

# Doyalson Wyee RSL Structure Plan <br> Transport Impact Assessment 

Prepared for:
Doyalson-Wyee RSL Limited
25 June 2019
The Transport Planning Partnership

# Doyalson Wyee RSL Structure Plan Transport Impact Assessment 

Client: Doyalson-Wyee RSL Limited
Version: Final
Date: 25 June 2019
TTPP Reference: 17395

Quality Record

| Version | Date | Prepared by | Reviewed by | Approved by | Signature |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Final | $25 / 06 / 19$ | O. Faiz | W. Johnson | W. Johnson |  |

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## 1 Introduction

This Planning Proposal has been prepared on behalf of Doyalson Wyee RSL Club (Club Ltd) to amend the Wyong Local Environmental Plan 2013 (WLEP 2013) for 80-120 Pacific Highway, Doyalson. This Planning Proposal is an Addendum to the Planning Proposal submitted for 100120 Pacific Highway in December 2018. The Club Ltd is seeking to diversify its offer to meet the needs of a growing local community and to enhance its unique landscape setting to create a new leisure experience - the Australian Resort. Therefore, this Planning Proposal seeks to facilitate the future redevelopment of the site for an integrated retail, recreation, community and residential precinct, centred around Doyalson Wyee RSL Club.

The current zoning permits a limited range of land uses, including rural and recreational uses. The Planning Proposal seeks to deliver a broader range of retail, recreation, community and residential uses. The Planning Proposal seeks to change the zoning from RU6 Transition to RE2 Private Recreation with an additional permitted use schedule that includes the following land uses contained in the concept plan.

An Indicative Concept Plan (Concept Plan) has been developed to support the Planning Proposal. The Concept Plan includes the following land uses:

- RSL Club
- Wellness and fitness centre
- Indoor and outdoor recreational facilities
- Tourism and accommodation
- Restaurants and cafes
- Medical centre
- Childcare centres
- Seniors living
- Residential
- Landscaping and open space.

The Transport Planning Partnership (TTPP) has undertaken an assessment of traffic and transport related aspects of the Planning Proposal for submission to Central Coast Council.

The report structure is set out as follows:

- Chapter 2 summarises the existing conditions around the site
- Chapter 3 provides a summary of the survey data collected
- Chapter 4 describes the Structure Plan proposal
- Chapter 5 assesses the car parking requirements of the site
- Chapter 6 provides an assessment of the traffic impacts of the Structure Plan
- Chapter 7 concludes and summarises the findings of the report.


## 2 Existing Transport Context

### 2.1 Site Description

The subject site is located in Doyalson, NSW within the local government area of Central Coast Council. The site includes an amalgamation of several properties including the Raw Challenge event site, and an existing fruit and vegetable farm (The Grove). The site properties are summarised in Table 2.1.

Table 2.1: Subject Site Properties

| Site Address | Existing Use |
| :---: | :---: |
| $49-65$ Wentworth Avenue and 80-90 Pacific Highway | Doyalson Wyee RSL Club and ancillary sporting fields |
| 100 Pacific Highway, Doyalson | Vegetation and ancillary land of the Doyalson Wyee <br> RSL Club |
| 110 Pacific Highway Doyalson | Raw Challenge event site |
| 120 Pacific Highway, Doyalson | Th fruit and vegetable farm) |

The location of the subject site in its regional context is shown in Figure 2.1.
Figure 2.1: Regional Contex


Basemap Source: Google Maps Australia

The subject site comprises three existing access points to the Pacific Highway as follows:

- 100 Pacific Highway
- 110 Pacific Highway
- 120 Pacific Highway.

The existing Doyalson Wyee RSL Club (herein RSL Club) is located at 80 Pacific Highway and has vehicular access from Wentworth Avenue.

The existing access points are shown in Figure 2.2.

Parking for the RSL Club is provided within an at-grade car park comprising 500 car spaces. Informal car parking is provided on the sports fields during events held at the club such as Food Festival and Raw Challenge event. The subject site and its local surrounds are shown in Figure 2.2.

Figure 2.2: Local Context


Base map Source: Nearmap

Key land uses surrounding the site include low-density residential housing, some commercial properties and significant bushland area. Notable establishments near the site include the Doyalson Fire Station at the corner of Wyee Road and Pacific Highway, a service station at the corner of Scenic Drive and Pacific Highway, and residential housing located on both sides of Wentworth Avenue, adjacent to the site.

### 2.2 Abutting Road Network

## Pacific Highway

Pacific Highway is a State Highway and provides a major north-south link through the Central Coast. Adjacent to the site, it is configured with two lanes in either direction plus auxiliary lanes at intersections.

South of the site, Pacific Highway connects with Doyalson Link Road, linking the highway to M1 Pacific Motorway, which generally runs parallel to Pacific Highway, between Newcastle and Sydney. Near the subject site, Pacific Highway has a posted speed limit of $80 \mathrm{~km} / \mathrm{hr}$.

## Wentworth Avenue

Wentworth Avenue is a two-way local road that provides access for the existing RSL Club and the adjacent residential dwellings to the local arterial road network. It is approximately nine metres in width, unmarked with unrestricted kerbside parking on both sides of the road. The RSL Club includes two access points along Wentworth Avenue.

### 2.3 Public Transport Conditions

Limited public transport services are available within the vicinity of the site. There are currently two bus stops on Pacific Highway within 150-metres of the site, and two bus stops on Wyee Road within 400-metres of the site. Seven bus services operate from these bus stops.

A summary of the existing bus services that operate in the vicinity of the site is detailed in Table 2.2, and shown in Figure 2.3 and Figure 2.4.

Table 2.2: Bus Services Summary

| Operator | Route \# | Route Description | Location of Service | Proximity to Site | Frequency (on-peak / offpeak) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Busways | 95 | Lake Haven to Morisse $\dagger$ | Doyalson Ambulance Station, Pacific Highway/Pacific Highway opposite Doyalson Ambulance Station | 150m | 1-2 trips on-peak/ 1-2 trips off-peak <br> No service (Sat, Sun, Pub Hol) |
|  | 95X | Lake Haven to Wyee |  |  | 1 trip on-peak/ <br> 1 trip off-peak <br> No service (Sat, Sun, Pub Hol) |
|  | 97 | Lake Haven to Mannering Park |  |  | Hourly on-peak/ hourly off-peak 1 trip (Sat, Sun, Pub Hol) |

$\begin{array}{c|c|c|c|c|c}\hline \text { Operator } & \text { Route \# } & \text { Route Description } & \text { Location of Service } & \begin{array}{c}\text { Proximity } \\ \text { to Site }\end{array} & \begin{array}{c}\text { Frequency (on-peak / off- } \\ \text { peak) }\end{array} \\ \hline & 98 & \begin{array}{c}\text { Lake Haven to Chain } \\ \text { Valley Bay }\end{array} & & & \begin{array}{r}30-60 \mathrm{~min} \text { on-peak/ } \\ 30-60 \mathrm{~min} \text { off-peak }\end{array} \\$\cline { 2 - 3 } $\left.1-5 \text { trips (Sat, Sun, Pub Hol) }\end{array}\right]$

Figure 2.3: Busways Existing Bus Route Map


Source: Busways [https://busways.com.au/sites/default/files/network_maps/WyongNetworkMap.pdf](https://busways.com.au/sites/default/files/network_maps/WyongNetworkMap.pdf)

Figure 2.4: Hunter Valley Bus Existing Bus Route Map


Source: CDC NSW [https://www.cdcbus.com.au/images/files/maps/hunter-valley/278-279-280-281_Map.pdf](https://www.cdcbus.com.au/images/files/maps/hunter-valley/278-279-280-281_Map.pdf)

### 2.4 Shuttle Bus Service

The RSL Club provides a courtesy bus service for pick up and drop off within the local area throughout the day, via a booking system. A summary of the existing courtesy bus services is shown in Table 2.3.

Table 2.3: Courtesy Bus Service Summary

| Service | Frequency | Stop Locations | Departure Time from Club |
| :---: | :---: | :---: | :---: |
| Pick up | Daily | Lake Haven, Charmhaven, Blue Haven, San Remo, Budgewoi, Buff Point | $\begin{aligned} & 2: 30 \mathrm{pm} \\ & \text { 5:15pm } \\ & \text { 7:30pm } \end{aligned}$ |
|  |  | Wyee, Wyee Point, Mannering Park | $\begin{aligned} & 3: 30 \mathrm{pm} \\ & 5: 55 \mathrm{pm} \\ & 8: 15 \mathrm{pm} \end{aligned}$ |
|  |  | Chain Valley Bay South, Chain Valley Bay North, Lake Munmorah | $\begin{aligned} & 4: 15 \mathrm{pm} \\ & 6: 45 \mathrm{pm} \\ & 8: 15 \mathrm{pm} \end{aligned}$ |
|  | Friday and Saturday only | Gwandalan, Summerland Point | $\begin{aligned} & 4: 15 \mathrm{pm} \\ & 6: 45 \mathrm{pm} \\ & 8: 15 \mathrm{pm} \end{aligned}$ |
|  |  | Woongarrah, Hamlyn Terrace (to Minnesota Road), Kanwal (to Craigie Avenue), Gorokan | 5:15pm 7:30pm |


| Service | Frequency | Stop Locations | Departure Time from Club |
| :---: | :---: | :---: | :---: |
| Drop off | Friday and Saturday only | Lake Haven, Charmhaven, Blue Haven, San Remo, Budgewoi, Buff Point, Woongarrah, Hamlyn, Terrace, Kanwal, Gorokan | $\begin{gathered} 9: 00 \mathrm{pm} \\ \text { 10:30pm } \\ \text { 12:00 am } \\ \text { 1:30 am } \\ \text { 3:00 am } \end{gathered}$ |
|  |  | Wyee, Wyee Point, Mannering Park, Chain Valley Bay South, Chain, Valley Bay North, Lake Munmorah, Gwandalan, Summerland Point | $\begin{gathered} 9: 45 \mathrm{pm} \\ 11: 15 \mathrm{pm} \\ 12: 45 \mathrm{am} \\ 2: 15 \mathrm{am} \end{gathered}$ |

### 2.5 Pedestrian and Cycle Infrastructure

Limited pedestrian facilities are provided around the site due to low pedestrian activity, as observed during site inspections. A short section of footpath spans across the service station located at the corner of Pacific Highway and Scenic Drive, and several adjacent properties along Pacific Highway 200-metres south west of the site. No formal pedestrian crossings are located near the RSL Club.

A shoulder cycle lane facility is currently provided along both sides of the Pacific Highway, passing the subject site. Cycling routes including off-road and on-road in the vicinity of the site are shown in the Roads and Maritime Services (Roads and Maritime) Cycleway Finder map as shown in Figure 2.5.

Figure 2.5: Surrounding Cycleways


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### 2.6 Crash History

The crash history of the most recent five-year period (June 2012 to June 2017) has been assessed for the roads adjoining the subject site. The study area includes approximately 2 km of Pacific Highway (between Wyee Road and Ruttleys Road), Wentworth Avenue and Barton Road.

A diagram showing the study area and the locations of the historical crashes is shown in Figure 2.6.

Figure 2.6: Crash History Map


Note: Each marker on the map can reference multiple crashes which have occurred at the same or general area.

The study area includes a total of eleven crashes. A breakdown of these crashes is summarised as follows:

- Three crashes occurred at the intersection of Wentworth Avenue/ Pacific Highway.
- All three of these crashes involved a vehicle turning right into Wentworth Avenue, colliding into either a through vehicle on Pacific Highway or a turning vehicle from Wentworth Avenue.
- The crashes occurred in 2013, 2015 and 2016 respectively.
- The crashes resulted in either a serious and/or moderate injury.
- There have been three southbound crashes along Pacific Highway involving:
transport planning
- a vehicle losing control on a bend
- a vehicle losing control and hitting an object off-road
- a vehicle colliding with another vehicle while undertaking a u-turn.
- There were five northbound crashes along Pacific Highway within the study area including:
- Two crashes involving a rear-end collision
- Two crashes involving a vehicle going off-road
- One crash involving a vehicle emerging from a driveway and colliding with through traffic.
- No fatalities were recorded in the five-year study period.
- There were 10 crashes resulting in injury and one resulting in property damage only.

The five-year historical crash data indicates that eleven crashes occurred within the study area (Pacific Highway between Wyee Road and Ruttleys Road and Wentworth Avenue). The crashes are all along the Pacific Highway and include three at the intersection of Pacific Highway and Wentworth Avenue. Notably, all three of these crashes involved a vehicle turning right from Pacific Highway into Wentworth Avenue indicating that there are safety issues with the existing Pacific Highway-Wentworth Avenue intersection configuration.

The data indicates that the intersection of Pacific Highway and Wentworth Avenue can be identified as a black spot site, based on the criteria set out by the Australian Government Black Spot Program.

The Australian Government Black Spot Program classifies black spot areas applicable for funding, as an area 3 km in length, with a minimum of three casualty crashes in the most recent five-year period.

On this basis, it is noted that the Pacific Highway, near the site contains safety risks associated with give-way movements, notably the right turn into Wentworth Avenue.

### 2.7 Strategic Context

### 2.7.1 M1 Pacific Motorway Upgrade (Tuggerah to Doyalson)

Roads and Maritime Services are currently constructing an upgrade of the M1 Pacific Motorway, between the Tuggerah and Doyalson interchanges. The upgrade includes the addition of traffic lanes (from two to three lanes in each direction) and capacity and safety improvements to the Doyalson interchange.

The upgrade is to cater for anticipated growth in traffic from population and employment growth in the Central Coast, Hunter and North Coast.

The project is expected to be completed in 2020.

### 2.7.2 Lake Munmorah Structure Plan

Several development projects are proposed in the Central Coast, including around Doyalson and specifically, along the Pacific Highway.

To assess the impact of these developments and coordinate for Lake Munmorah implementation, Central Coast Council is preparing a Structure Plan which is planned for public exhibition in late 2018. The Lake Munmorah Structure Plan includes an assessment of the area between Ruttleys Road and Chain Valley Bay Road as shown in Figure 2.7.

Figure 2.7: Lake Munmorah Structure Plan Area


The following transport assessment has been undertaken to assess the impact of the Doyalson Wyee RSL Structure Plan between Wyee Road and Ruttleys Road. Background traffic growth of 1.8 to 2.0 per cent per annum along Pacific Highway has been adopted from Roads and Maritime Services' Sydney Traffic Forecasting Model, which is assumed to take into account traffic growth from the Lake Munmorah development.

### 2.7.3 Pacific Highway/ Wyee Road/ Scenic Drive Assessment

Roads and Maritime Services are currently undertaking a detailed assessment of the intersection of Pacific Highway, Wyee Road and Scenic Drive. GHD has been commissioned to model and assess the operation of the intersection to improve the intersection's safety and performance. The future layout of the Pacific Highway, Wyee Road and Scenic Drive
intersection has not been released by Roads and Maritime Services and therefore has not been assessed by TTPP as part of this transport assessment.

## 3 Traffic Survey Data

Several traffic surveys were undertaken as part of the transport assessment to record conditions during the typical operation of the RSL Club and also during special events.

Traffic surveys were undertaken on a typical day on Friday 2 February 2018 and Saturday 3 February 2018. Additional 'event' surveys were commissioned to record the transport characteristics of the monthly Food Festival event on Saturday 3 March 2018, between 4pm and 9pm and the Raw Challenge event which was held on 24 and 25 March 2018.

The following surveys were undertaken for each survey period:

- Intersection furning movement counts survey
- Car occupancy survey
- Parking demand survey.

The survey results are presented below.

### 3.1 Traffic Volumes

### 3.1.1 Road Network Peak Volumes

Based on the survey data, the key road network peak hours are as follows:

- Friday 7:30am-8:30am.
- Friday 4:45pm-5:45pm.
- Saturday 11:00am-12:00pm.

The turning volumes observed during the above peak hours, are illustrated in Figure 3.1.

Figure 3.1: Existing Road Network Peak Volumes
2018 EXISTING ROAD NETWORK PEAK VOLUMES
10 (10) [10] = AM (PM) [SAT] PEAKS
AM PEAK $=7: 30-8: 30$
PM PEAK = 16:45-17:45
SAT PEAK $=11: 00-12: 00$


### 3.1.2 Typical RSL Club Traffic

Traffic at the RSL Club's access points on Wentworth Avenue were also surveyed. The survey data shows that the peak hour at the RSL Club to be during the following hours:

- Friday $5: 30 \mathrm{pm}$ to $6: 30 \mathrm{pm}$ with 313 two-way vehicle movements per hour entering/ exiting the site
- $\quad$ Saturday $5: 45 \mathrm{pm}$ to $6: 45 \mathrm{pm}$ with 364 two-way vehicle movements per hour entering/ exiting the site.

The hourly profile of vehicle movements at the RSL Club during typical operating conditions on a Friday and Saturday is summarised in Figure 3.2 and Figure 3.3 respectively.

Figure 3.2: $\quad$ Site Peak Volumes (Friday)


Figure 3.3: $\quad$ Site Peak Volumes (Saturday)


### 3.1.3 Food Festival Traffic

The RSL Club traffic generation during the Food Festival event held in March has been collected and is summarised in Figure 3.4.

The data indicates that traffic generated by the RSL Club and the Food Festival event generate up to 641 two-way vehicle movements per hour, with a site peak hour of 5:45pm to 6:45pm.

Figure 3.4: $\quad$ Site Peak Volumes (Food Festival)


### 3.1.4 Raw Challenge

A Raw Challenge event was held on Saturday 24 and Sunday 25 March and ran between 8:30am and $4: 00 \mathrm{pm}$. Parking for the event is accommodated within the RSL Club car park and informal parking areas provided on-site.

Traffic surveys undertaken during the Raw Challenge event indicate a peak traffic generation of 408 two-way vehicle movements per hour, occurring from 11:45am to 12:45pm on the Saturday. On Sunday, the traffic generation is lower, with a peak of 364 vehicle movements per hour at 1:30pm. This traffic generation includes traffic generated by the Raw Challenge event and typical RSL Club visitors.

The traffic generation profile of the event is shown in Figure 3.5.
Figure 3.5: $\quad$ Site Peak Volumes (Raw Challenge)

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### 3.2 Car Occupancy

A car occupancy survey was undertaken of vehicles entering and exiting the RSL Club to understand the car sharing patterns to the site. Car occupancy surveys were undertaken during a typical Saturday operation and during the Food Festival and Raw Challenge events held in March.

The survey results indicate the following average car occupancy rates:

- Typical Saturday: 1.9 persons per vehicle
- Food Festival: 2.1 persons per vehicle
- Raw Challenge: 2.2 persons per vehicle.


### 3.3 Parking Demand

### 3.3.1 Typical Demand

A survey of parking restrictions and demand was undertaken of the RSL Club during the noted survey dates. The survey identified a car parking supply of around 500 spaces within the on-site car park, available during the typical operation of the club.

On a typical Friday, the peak parking demand was recorded as 116 vehicles ( 23 per cent occupancy) during the morning peak period and 375 vehicles ( 75 per cent occupancy) during the evening peak period as detailed in Figure 3.6. The peak parking demands were recorded at 8:45am and 7:15pm respectively.

Figure 3.6: Parking Demand (Friday)


On a typical Saturday, two peaks are observed including an afternoon peak with 297 vehicles at 1:30pm, and an evening peak with a parking demand of 361 vehicles ( 72 per cent occupancy) occurring at $6: 45 \mathrm{pm}$. The parking demand profile over the surveyed Saturday is detailed in Figure 3.7.

Figure 3.7: Parking Demand (Saturday)


### 3.3.2 Food Festival Demand

A car parking demand survey was undertaken during the Food Festival event held in March on-site.

In addition to the formal car parking area, informal car parking was also provided on the sporting fields for the event, to accommodate the additional parking demand generated by the special event.

The peak parking demand during the Food Festival event was 738 vehicles at 7:00pm, as shown in Figure 3.8.

TTPP notes that more than 500 informal car spaces could be provided on the sports fields surrounding the Club. During the Food Festival event held in March some 238 cars were parked on the sports fields.

Figure 3.8: $\quad$ Parking Demand (Food Festival)


### 3.3.3 Raw Challenge

A car parking demand survey was undertaken during the Raw Challenge event held on Saturday 24 March and Sunday 25 March. Parking for the Raw Challenge event is accommodated within the RSL site car park. Similar, to the Food Festival event, the additional parking demand was catered for on the sporting fields.

The peak parking demand during the Raw Challenge event was 714 vehicles at noon on Saturday, as shown in Figure 3.9. On this basis, some 214 cars were informally parked on the sporting fields on Saturday.

Peak parking demand on the Sunday was 590 vehicles at 12:15pm, indicating around 90 vehicles were informally parked on the sporting fields.

Figure 3.9: Parking Demand (Raw Challenge)

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## 4 Structure Plan

### 4.1 Land Uses

Doyalson-Wyee RSL Limited has developed a Structure Plan for their large landholding in Doyalson, NSW comprising an amalgamation of several properties. The proposed Structure Plan for the RSL Club aims to diversify its services to protect the Club's longevity against potential changes to its tenure and gaming regulations. The Structure Plan includes several new land uses including a new RSL Club as detailed in Table 4.1.

Table 4.1: Land Use Summary

| Land Use | Existing Floor Area (m²) | Proposed Floor Area (m²) |
| :---: | :---: | :---: |
| RSL Club |  |  |
| - Club | 7,424 | 7,500 |
| - Dining Area/Food Court |  | 3,390 |
| - Function Room |  | 750 (500-pax) |
| Hairdresser |  | 120 |
| Dry Cleaner |  | 120 |
| Health \& Wellness Precinct |  |  |
| - Gym | 1,679 | 3,000 |
| - Spa |  | 350 |
| - Swimming Pool |  | 1,000 |
| - Medical Clinic |  | 4,830 |
| - Physio |  | 150 |
| Motel/hotel |  | 7,641 (102 units \& 72 villas) |
| Childcare |  | 5,632 (360 places) |
| Fast Services |  |  |
| - Fast Food Outlets |  | 1,055 ( $4 \times 60$ seats ea) |
| - Car Wash |  | 30 |
| - Petrol Station |  | 590 |
| Broadcare Leisure \& Recreational |  |  |
| - Raw Challenge | 17,000 | 4.7 km course |
| - Sporting Fields | 50,000 | NA |
| - Go Kart |  | 6,938 |
| - Recreational Warehouse |  | 5,858 |
| Residential Development |  |  |
| - Residential Dwellings |  | 137 lots/dwellings |
| - Senior Living |  | 214 lots/dwellings |

### 4.2 Structure Plan Staging

The proposed Doyalson Wyee RSL Structure Plan is to be implemented in stages over a period of 20 years.

For the purposes of this traffic assessment, the development has been indicatively split into two key stages; Stage 1 incorporates the development to be implemented over the first ten years and Stage 2 incorporates the development to be implemented after Stage 1 in the following ten years.

### 4.2.1 Stage 1 Development

The Stage 1 development would include the following land uses that are programmed to be implemented in stages over the next 10-years:

- Stage 1 RSL Club (4,240m²) and function room (300m²/ 200-person capacity))
- Senior Living (100 dwellings assumed for the purposes of this report)
- Fast Services (Fast Food Outlets, Car Wash, Petrol Station)
- Childcare Centre
- Hotel
- Health and Wellness Precinct (Gym)

Stage 1 may comprise partial build of the senior living lots, however, the number is not confirmed at this stage. For the purposes of TTPP's traffic analysis, 100 of the 214 dwellings has been conservatively assumed for Stage 1.

In addition, the proposed signalised site access would be constructed.

### 4.2.2 Stage 2 Development

The Stage 2 development would include the following land uses and is proposed to commence in 10 to 15 years' time, following the completion of Stage 1:

- Stage 2 RSL Club (3,260m²) and function room (450m²/ 300-person capacity)
- Health and Wellness Precinct (spa, swimming pool, medical clinic, physio)
- Leisure and Recreational (Go Kart, recreational warehouse)
- remaining Senior Living
- Residential (137 dwellings).


### 4.3 Site Layout and Access

The existing RSL Club is located on the property of 49-65 Wentworth Avenue and 80-90 Pacific Highway. The proposed Structure Plan including RSL Club will be relocated within 100-120 Pacific Highway, with a new signalised access located at 100 Pacific Highway, approximately 400m north of Wentworth Avenue.

A strip of crown land separates 110 Pacific Highway and 120 Pacific Highway. As such, the recreational land uses located on 120 Pacific Highway are to be accessed separately via the existing left-in/left-out access point. This existing left-in/left-out access to 120 Pacific Highway would be formalised to be compliant with Austroads guidelines. Negotiation for an easement across crown land are ongoing to enable a vehicular link between 110 and 120 Pacific Highway. The proposed site layout is shown in Figure 4.1.

Figure 4.1: $\quad$ Site Masterplan Layout


[^1]
### 4.4 Road Widths

The internal road layout will feature a hierarchy of roads. A summary of proposed road widths is detailed in Table 4.2. A layout of the indicative road network is shown in Figure 4.2.

Table 4.2: Road Widths

| Road Type | Carriageway | Verge | Median | Total Road Reserve | DCP Subdivision Road Width Requirement |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Main Road | $14 m$ <br> (7m in each direction) | $4.5 \mathrm{~m} \times 4.5 \mathrm{~m}$ | 4.5m | 27.5m | Collector Street: <br> Verge: 4.5m Carriageway: 9.6 m or 12 m with bus route Total reserve: 18.6 m |
| Local Road | 7 m | $3.5 \mathrm{~m} \times 3.5 \mathrm{~m}$ | NA | 16.5m | Local Street: <br> Verge: 4.5 m Carriageway: 7.6 m or 12 m with bus route Total reserve: 16.6 m |
| Boundary | 8m | $3.5 \mathrm{~m} \times 1.0 \mathrm{~m}$ | NA | 12.5m |  |
| Access Street (internal to Residential and Senior Living) | 6 m | $3.0 \mathrm{~m} \times 3.0 \mathrm{~m}$ | NA | 12.0m | Access Street: <br> Verge: 4.5m [1] <br> Carriageway: 5.5m <br> Total reserve: 14.5 m |

[1] Verge width of 4.5 m required at development frontages. Otherwise a minimum one metre verge is required.

Figure 4.2: Indicative Road Network Layout


The proposed road network has been designed with consideration for minimum road width requirements detailed in Part 4 (Subdivision) of Council's DCP. Road widths are generally wider than Council's minimum requirements with the exception of verge widths, which are 3.5 m along active frontages, where Council requires a 4.5 m verge.

Consultation with Council has been undertaken on the proposed road widths. Council have stated that as the proposed roads are not public roads, verge widths may be varied provided that adequate allocation is provided for services.
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### 4.5 Engagement with Authorities \& Community

### 4.5.1 Roads and Maritime Services/ Central Coast Council

Consultation has been undertaken with Roads and Maritime Services (Roads and Maritime) on the following matters:

- proposed access arrangements
- trip generation rates for the proposed land uses
- future road upgrades.

Consultation included meetings with RMS on Tuesday 17 April 2018 and 29 November 2018 and email correspondence.

Roads and Maritime has given in-principle support for the proposed access arrangements including a signalised access intersection with Pacific Highway and a secondary access point (left on/ left out) from 120 Pacific Highway.

Trip rates for each land use as detailed in Section 6.1, have been approved by Roads and Maritime and adopted as part of our transport assessment.

Roads and Maritime has expressed concerns regarding the impact of the Lake Munmorah Plan on the Pacific Highway corridor and have highlighted a preference to develop an area wide study to include the proposed development. On this basis, TTPP consulted with Central Coast Council (Council) via email in August 2018.

Council has indicated that the Lake Munmorah Structure Plan is too far along in its progress to include the proposed development site. On this basis, the traffic assessment for the proposed development (as detailed in this report) is required to be assessed independently.

### 4.5.2 Community Consultation

Community consultation has indicated general support for the proposed development, noting a reduction in turning traffic volumes at the intersection of Pacific Highway and Wentworth Avenue.
transport planning

## 5 Car Parking Requirements

### 5.1 Assumptions

The car parking requirement for existing/reinstated land uses has been estimated based on the existing parking demand for the site.

The existing site includes an RSL Club with several food outlets, community services and a Fitness Centre/ Gym. Based on a total floor area of $9,103 \mathrm{~m}^{2}$, the existing parking demand per $100 \mathrm{~m}^{2}$ of floor area is summarised in Table 5.1.

Table 5.1: Existing Parking Demand

| Time Period | Peak Parking Demand | Parking Rate (per $100 \mathrm{~m}^{2}$ ) |
| :---: | :---: | :---: |
| Fri $(19: 15)$ | 375 | 4.1 |
| Sat $(18: 45)$ | 361 | 4.0 |

Table 5.1 shows that the existing site generates a peak parking demand of 4.1 spaces per $100 m^{2}$ of gross floor area. On this basis, the peak parking demand rate has been adopted for the following retained land uses:

- RSL Club
- Dining \& Food Court Area
- Gym.

For new land uses, the parking requirement has been sourced from Wyong Shire Development Control Plan 2013 (DCP) where possible. The following land uses have been determined based on the DCP, with the applied DCP category identified in parentheses.

- Function Room (Restaurant and Function Centre)
- Hairdresser/ Dry Cleaner (Shops)
- Spa (Shops)
- Swimming Pool (Swimming Pools)
- Medical Clinic/ Physio (Medical Centre)
- Motel/ Hotel (Motel)
- Fast Food Outlet (Take Away Food \& Drink)

Where the DCP does not provide guidance, additional sources have been referenced including the following reports in relation to the 'Childcare' and 'Car Wash' land uses:

- Child Care Planning Guideline, NSW Planning and Environment, 2017
- Proposed Carwash, 412-416 Liverpool Rd, Croydon, D\&H Kane investments, 2006
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No suitable survey data could be identified to determine the typical parking demands of 'recreational warehouse' (e.g. trampoline park, rock climbing, laser tag etc) or 'Go Kart' facility. As such, TTPP has reviewed the parking supply of existing sites which include comparable uses.

A desktop study of the Luddenham Raceway - Go Karting, Paintball \& Motorsport Park, has identified a parking supply of 80 spaces to accommodate combined Paintball and Go Kart services. It is assumed the provision allows for 40 spaces each for the Paintball and Go Kart facility. On this basis, a similar parking supply has been assumed for the proposed Go Kart use, that is, 40 spaces.

In relation to the recreational warehouse which may have a variety of uses, two sites were reviewed including the Red Alert Laser Tag and Indoor Go Kart facility at Warners Bay, and the Inflatable World Charmhaven, Charmy Indoor Sports and Millennium Health Club facility at Charmhaven. The two sites include 40 to 60 spaces for an estimated floor area of $2,800 \mathrm{~m}^{2}$ to $2,300 \mathrm{~m}^{2}$ respectively. On this basis, a parking rate of 50 spaces per $2,500 \mathrm{~m}^{2}$ or two spaces per $100 \mathrm{~m}^{2}$ has been applied to the proposed recreational warehouse.

It is expected that any additional or atypical parking demands related to special events (e.g. Raw Challenge, Food Festival) would be catered for under a special event management plan.

### 5.1.1 Senior Living

The proposal for the Senior Living component of the Structure Plan includes 214 dwellings.

The State Environmental Planning Policy (Housing for Seniors or People with a Disability) 2004 states that self-contained dwellings for seniors are to have a minimum parking requirement of 0.5 spaces per bedroom where the development application is made by a person other than a social housing provider. However, at this Planning Proposal stage the number of bedrooms per dwelling is undetermined. On this basis, an average of two bedrooms per dwelling has been assumed, indicating a parking requirement of one space per dwelling.

### 5.2 Parking Requirement

Based on the assumptions detailed in Section 5.1, the parking requirement for the proposed Structure Plan is $\mathbf{1 , 4 8 4}$ spaces for the commercial and recreational uses and 100 spaces for the senior living land use as summarised in Table 5.2.
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Table 5.2: Parking Assessment

| Land Use | Size ( $\mathrm{m}^{2}$ GFA) | Car Parking Rate/ Demand | Parking (Spaces) |
| :---: | :---: | :---: | :---: |
| RSL Club |  |  |  |
| - Club | 7,500 | 4.1 per 100sqm | 309 |
| - Dining Area/Food Court | 3,390 | 4.1 per 100sqm | 140 |
| - Function Room (750sqm) | 500 people | 15 spaces per 100 sqm or 1 space per 3 seats whichever is greater | 167 |
| Hairdresser | 120 | 1 space per 20sqm GFA | 6 |
| Dry Cleaner | 120 | 1 space per 20sqm GFA | 6 |
| Subtotal |  |  | 627 |
| Health \& Wellness Precinct |  |  |  |
| - Gym | 3,000 | 4.1 per 100sqm | 124 |
| - Spa | 350 | 1 space per 20sqm GFA | 18 |
| - Swimming Pool | 1,000 | 30 spaces per 500sqm (water area only) | 60 |
| - Medical Clinic (4,830sqm GFA) | 32 rooms | 3 spaces per consulting room +1 space per | 107 |
| - Physio (150sqm) | 2 | employee | 7 |
| Subtotal |  |  | 315 |
| Motel/hotel (102 units+72 villas) | 174 units | 1 space per unit + 1 space for manager + 1 space per 2 employees [1] | 200 |
| Childcare | 360 places | 1 space per 4 children | 90 |
| Fast Services |  |  |  |
| - Fast Food Outlet (1,055sqm GFA) | 240 seats | 1 space per 3 seats | 80 |
| - Car Wash | 30 | 1 employee space + 2.5 vehicle queveing area per lane | 4 |
| - Petrol Station | 590 | 5 spaces per 100m2 GFA store [2] | 10 |
| Subtotal |  |  | 94 |
| Broadcare Leisure \& Recreational |  |  |  |
| - Raw Challenge | 4.7 km course | Special Event Management Plan | NA |
| - Go Kart | 6,938 |  | 40 |
| - Recreational Warehouse | 5,858 | 2 spaces per $100 \mathrm{~m}^{2}$ | 117 |
| Subtotal |  |  | 157 |
| Residential/ Senior Living |  |  |  |
| - Residential | 137 | 1 space per dwelling 1-3 bedrooms and 2 spaces per dwelling 4+ dwellings | 137 |
| - Senior Living Dwellings | 214 | 0.5 space per bedroom (2 bedrooms per dwelling assumed) | 214 |
| Subtotal |  |  | 351 |
| Total (commercial/ recreational) |  |  | 1,484 |
| Total (incl. Residential/ Senior Living) |  |  | 1,835 |

[1] 25 employees assumed for the Motel/Hotel
[2] $200 \mathrm{~m}^{2}$ convenience store assumed as part of the Petrol Station

The proposed parking supply on-site would aim to accommodate the peak usage of the overall site on a typical day. However, the peak parking demand outlined in Table 5.2 is the "worst case" and does not consider opportunity to share parking supply between the commercial and recreational land uses.

For example, typically, the following uses have peak parking demands outside of the peak demand of the RSL Club and associated dining facilities (6:30-7:00pm weekdays and weekends), in particular:

- a childcare centre's peak demand occurs in the morning (7am-9am) and afternoon (2:30-4:00pm) weekday peaks
- leisure and recreational activities peak demand occurs during the weekend (11am$3 \mathrm{pm})$. Based on existing recreational facilities nearby, the recreational facilities are expected to operate during business hours on the weekdays and weekends.
- retail (hairdressers and drycleaner) is expected to be open only during typical business hours on the weekdays and weekends.

While the operating hours of the land uses are still to be determined, a parking accumulation assessment has been undertaken based on typical hours of operation as detailed in Table 5.3.

Based on the parking accumulation assessment in Table 5.3, the peak parking demand of the site is expected to occur after 6pm on a weekday, with up to 75 per cent of the DCP parking requirement to be utilised i.e. 1,108 spaces, not including the residential/ senior living dwellings.

Table 5.3: Car Parking Accumulation Assessment

| Land Use | Parking Requirement (Spaces) | Percent Parking Demand by Time Period |  |  |  |  |  |  |  |  |  | Parking Req' during Peak Demand (Spaces) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $\begin{aligned} & \text { E } \\ & 0 \\ & \text { i } \\ & \text { E } \\ & \text { ㅇ } \\ & \text { in는 } \end{aligned}$ | $\begin{aligned} & \hline \text { 틍 } \\ & \text { N } \\ & \stackrel{1}{1} \\ & \frac{1}{0} \\ & \circ \\ & \hline \end{aligned}$ |  |  | 틍 응 등 은 | $\begin{aligned} & \frac{\varepsilon}{0} \\ & \frac{1}{\sigma} \\ & \alpha \\ & \bar{b} \\ & \sim \end{aligned}$ |  | $\begin{aligned} & \varepsilon \\ & \stackrel{0}{2} \\ & \stackrel{1}{-} \\ & \stackrel{\rightharpoonup}{0} \\ & \hline \sim \sim \end{aligned}$ |  | $\begin{aligned} & \varepsilon \\ & \vdots \\ & 0 \\ & \vdots \\ & \vdots \\ & \vdots \\ & 0 \\ & \hline \end{aligned}$ |  |
| RSL Club |  |  |  |  |  |  |  |  |  |  |  |  |
| - Club | 309 | 25\% | 25\% | 50\% | 75\% | 100\% | 25\% | 50\% | 50\% | 75\% | 100\% | 309 |
| - Dining Area/Food Court | 140 | 25\% | 25\% | 50\% | 50\% | 100\% | 25\% | 50\% | 50\% | 75\% | 100\% | 140 |
| - Function Room (750sqm) | 167 | 0\% | 50\% | 50\% | 100\% | 100\% | 50\% | 100\% | 100\% | 100\% | 100\% | 167 |
| Hairdresser | 6 | 0\% | 50\% | 50\% | 50\% | 10\% | 50\% | 100\% | 100\% | 50\% | 10\% | 1 |
| Dry Cleaner | 6 | 0\% | 50\% | 50\% | 50\% | 10\% | 50\% | 100\% | 100\% | 50\% | 10\% | 1 |
| Subtotal | 627 |  |  |  |  |  |  |  |  |  |  | 617 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Health \& Wellness Precinct |  |  |  |  |  |  |  |  |  |  |  |  |
| - Gym | 124 | 100\% | 50\% | 50\% | 50\% | 90\% | 75\% | 75\% | 75\% | 75\% | 75\% | 111 |
| - Spa | 18 | 0\% | 75\% | 75\% | 75\% | 10\% | 75\% | 100\% | 100\% | 75\% | 10\% | 2 |
| - Swimming Pool | 60 | 100\% | 50\% | 50\% | 75\% | 80\% | 50\% | 100\% | 100\% | 50\% | 25\% | 48 |
| - Medical Clinic (5,000sqm GFA) | 107 | 100\% | 100\% | 100\% | 100\% | 10\% | 100\% | 100\% | 100\% | 50\% | 10\% | 11 |
| - Physio (150sqm) | 7 | 100\% | 100\% | 100\% | 100\% | 10\% | 100\% | 100\% | 100\% | 50\% | 10\% | 1 |
| Subtotal | 315 |  |  |  |  |  |  |  |  |  |  | 173 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Motel/Hotel | 200 | 100\% | 50\% | 50\% | 50\% | 100\% | 50\% | 50\% | 50\% | 50\% | 100\% | 200 |
| Childcare | 90 | 100\% | 25\% | 25\% | 100\% | 10\% | 0\% | 0\% | 0\% | 0\% | 0\% | 9 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |

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### 5.3 Adequacy of Parking Supply

The senior living and residential component are to be separate from the commercial site and is expected have its own private parking provisions. Publicly accessible car parking is to be provided for the commercial and recreational uses with parking to be shared.

Based on the parking demand and accumulation assessment shown in Table 5.3, the commercial and recreational uses would generate a peak parking demand of 1,108 spaces.

The proposed development is to provide 1,295 spaces distributed over several at-grade car parks and on-street kerbside spaces, which would satisfy the peak parking demand of the site. In addition, an overflow car park for events is proposed accommodating an additional 104 spaces. The car parking layout is shown in Figure 5.1.

Figure 5.1: $\quad$ Site Car Park


Base Source: Urbis, 19/16/19
Note: Side parking refers to on-street parking bays on main access road

### 5.4 Accessible Car Parking

The DCP states that accessible parking should be provided based on the Building Code of Australia (BCA) guide. On this basis, the RSL Club and associated commercial and
recreational facilities are generally classified as a Class 6 development, the childcare centre is classed as 9b, car wash as Class 8 and medical services as Class 5 in the BCA guide.

On this basis, the site requires a minimum accessible parking provision of 29 spaces as summarised in Table 5.4.

Table 5.4: Accessible Parking Assessment

| Land Use | DCP Parking Requirement | BCA Parking Rate | Source | Parking (Spaces) |
| :---: | :---: | :---: | :---: | :---: |
| RSL Club (club, dining, function rooms, retail) | 627 | 1 space per 50 spaces | Class 6 as per BCA | 12.5 |
| Health \& Wellness Precinct |  |  |  |  |
| - Gym/ Spa/ Swimming Pool | 201 | 1 space per 50 spaces | Class 6 as per BCA | 4.0 |
| - Medical Clinic/ Physio | 114 | 1 space per 100 spaces | Class 5 as per BCA | 1.2 |
| Subtotal |  |  |  | 5.2 |
| Hotel | $\begin{gathered} \text { TBC } \\ (\text { See Note 1) } \end{gathered}$ | Total \#car spaces multiplied by the percentage of accessible units or bedrooms | Class 1B/ 3 in BCA | 4.0 |
| Childcare | 90 | 1 space per 50 spaces | Class 9b as per BCA | 1.8 |
| Fast Services | 94 | 1 space per 50 spaces | Class 6 as per BCA | 1.8 |
| Broadcare Leisure \& Recreational | 157 | 1 space per 50 spaces | Class 6 as per BCA | 3.1 |
| Total |  |  |  | 28.4 |

[1] DA to confirm number of the adaptable units within the hotel. Four accessible spaces assumed for the purposes of this report

## 6 Traffic Impac†

### 6.1 Traffic Generation

The traffic generation for existing/reinstated land uses has been estimated based on the existing traffic generation rate for the site.

For new land uses, typical traffic generation estimates for the proposed development have been sourced from Roads and Maritime Service's Draft Guide to Traffic Generating Developments (2018) and the updates in the Technical Direction TDT2013/04a. In addition, the following additional sources have been referenced where appropriate:

- Trip Generation Surveys, Medical Centres, Analysis Report, TEF Consulting, 2015
- Mixed Use Development, 462-482 Swan Street, Richmond, GTA Consultants, 2015
- Sydney Polo Club, Richmond Wedding Venue, GTA Consultants, 2016
- Proposed Carwash, 412-416 Liverpool Road, Croydon, K\&H Kane Investments Pty Ltd, 2006
- Trip Generation Surveys at Child Care Centres Analysis Report, TEF Consulting, 2015

The above sources and traffic generation rates as detailed below, have been reviewed and confirmed by Roads and Maritime Services (Roads and Maritime).

The proposed traffic generation rates for the Doyalson Wyee Club Structure Plan are detailed in the following sections.

### 6.1.1 Existing Club Uses

The existing site includes an RSL Club with several food outlets, community services and a Fitness Centre/ Gym. Based on a total floor area of 9,103m², the existing traffic generation rates for the site is summarised in Table 6.1.

Table 6.1: Existing Club Trip Rates

| Time Period | Site Traffic (vph) | Existing Trip Rate (per 100m2) |
| :---: | :---: | :---: |
| Road Network Peak | 120 |  |
| Fri AM (7:30-8:30) | 280 | 1.3 |
| Fri PM (16:45-17:45) | 231 | 3.1 |
| SAT (11:00-12:00) |  | 2.5 |
| Site Peak | 313 | 3.4 |
| Fri PM (17:30-18:30) | 364 | 4.0 |
| SAT (17:45-18:45) |  |  |

The above traffic generation summary highlights that the Club generated a maximum of 313 vehicles per hour during the Friday afternoon peak period (17:30pm - 18:30pm). It is noted that the road network peak period occurs between 16:45pm - 17:45pm which is around one hour earlier than the peak hour traffic generation of the Club, as shown in the hourly site traffic generation profiles in Figure 3.2 and Figure 3.3.

In theory, the traffic generation in the network peak hour could be reduced, however, TTPP has assessed the worst-case scenario for the evening peak hour i.e. peak road networks and peak club operation occurring simultaneously. As such, the site peak traffic generation rate ( 3.4 trips per $100 \mathrm{~m}^{2}$ ), has been adopted for the future traffic generation during the evening road network peak hour.

On this basis, the above existing trip rates are adopted for the following retained land uses:

- RSL Club
- Dining \& Food Court Area.


### 6.1.2 Gym

Traffic generation rates for the Gym and physio have been obtained from recent Roads and Maritime surveys as detailed in the report "Trip Generation and Parking Demand Surveys of Gymnasiums Data and Analysis Report" (TEF Consulting, 2015). The adopted rates relate to an average of three comparable survey sites including Oatley, Willoughby and Riverwood in NSW. The three comparable sites have an average peak traffic generation of 4.3 trips per 100sqm.

Traffic generation for a gym is understood to be relatively lower during the weekday morning and Saturday road network peaks compared to the weekday afternoon road network peak. On this basis, the above rate has been adopted for the PM peak period, while the AM peak is anticipated to generate 75 percent of the PM traffic generation and the Saturday peak is 50 per cent of the PM peak period.

To summarise, the following rates have been adopted for the proposed Gym:
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- $\quad$ AM peak hour vehicle trips $=3.2$ trips per 100 sqm
- PM peak hour vehicle trips $=4.6$ trips per 100 sqm
- SAT peak hour vehicle trips $=2.2$ trips per 100 sqm.


### 6.1.3 Function Room

The Roads and Maritime Guide does not have a trip rate for function rooms. Traffic generation has instead been determined based on a traffic survey of a wedding event held in a comparable location. The survey results indicated that $83 \%$ of people arrived by private vehicle with an average car occupancy of 2.2 persons per vehicle, and $28 \%$ arrived by bus. Approximately $75 \%$ of vehicles arrived within the hour leading up to the event and included a $77 \%$ inbound/ $23 \%$ outbound distribution of traffic.

Noting that the RSL Club includes provision of a bus service, the rates identified in the survey, have been applied to the proposed function centres as follows:

- AM peak hour vehicle trips = NA (events are not likely to occur during the morning road network peak)
- PM peak hour vehicle trips = 83\% car usage + 2.2 car occupancy $+75 \%$ arrival rate $=141$ vehicles per hour
- SAT peak hour vehicle trips $=83 \%$ car usage +2.2 car occupancy $+75 \%$ arrival rate $=141$ vehicles per hour

However, the above estimate assumes that both proposed function centres with capacity for 250 persons each, would have an event at the same time generating 500 people. In addition, both these function centres would reach maximum capacity within the same hour. TTPP notes that this occurrence is very unlikely and have therefore applied a $50 \%$ reduction to the traffic generation estimate i.e. ( 2 rooms of 125 people arriving in the same hour is more realistic on average).

Based on the above, the two function centres have been estimated to generate up to 71 vehicles per hour.

### 6.1.4 Hairdresser/ Dry Cleaner

The proposed hairdresser and dry cleaner are considered a retail land use. As such, the Roads and Maritime rate for a speciality store has been applied:

- AM peak hour vehicle trips $=50 \%$ of the PM peak traffic generation estimate
- PM peak hour vehicle trips $=4.6$ trips per 100 sqm
- SAT peak hour vehicle trips $=5.6$ trips per 100 sqm.


### 6.1.5 Spa

Roads and Maritime do not have any trip rates for a Spa land use. However, the proposed spa $\left(350 \mathrm{~m}^{2}\right)$ is a relatively small land use and is expected to be ancillary to the Gym and RSL Club. For the purposes of estimating the additional traffic generation from the Spa, the traffic generation rate for the existing Club/ Gymnasium has been applied:

- AM peak hour vehicle trips = 1.3 trips per 100sqm
- PM peak hour vehicle trips $=3.4$ trips per 100 sqm
- SAT peak hour vehicle trips $=2.5$ trips per 100 sqm


### 6.1.6 Swimming Pool

Roads and Maritime do not have any trip rates for a Swimming Pool. Rates for a swimming pool have instead been sourced from surveys undertaken at the State Swim Centre in Urley and Seaford (May 2010) as referenced in the report "Mixed Use Development, 462-482 Swan Street, Richmond" (GTA Consultants, 2015). On this basis, a rate of 6.1 movements per swimming lane has been adopted, assuming 8 lanes per pool.

### 6.1.7 Medical Clinic/ Physio

Traffic generation rates for the Medical Clinic and physio have been obtained from recent Roads and Maritime surveys as detailed in the report "Trip Generation Surveys, Medical Centres, Analysis Report" (TEF Consulting, 2015). The adopted rates relate to Regional Sites, and are as follows:

- AM peak hour vehicle trips $=3.1$ trips per 100 sqm
- $\quad$ PM peak hour vehicle trips $=2.5$ trips per 100 sqm
- SAT peak hour vehicle trips = 11 trips per 100 sqm


### 6.1.8 Motel/ Hotel

The rate provided in the Roads and Maritime guide has been applied for the AM, PM and SAT peak periods for the Motel/Hotel use, with 0.4 trips per unit.

The hotel trip rate will be lower as some hotel patrons will frequent the Club for dinner and functions etc. i.e. linked trips.

### 6.1.9 Childcare Centre

The Roads and Maritime Guide states the following traffic generation rates for a child care centre:

- AM peak hour vehicle trips $=0.64$ trips per child
transport planning
- PM peak hour vehicle trips $=0.34$ trips per child
- SAT peak hour vehicle trips = NA.


### 6.1.10 Fast Food Outlet

The Roads and Maritime guide states typical traffic generation for several fast food chains including McDonalds, Kentucky Fried Chicken (KFC) and Hungry Jacks. Noting that four fast food outlets are proposed on site, the traffic generation volumes for one McDonalds (McD), one KFC and Hungry Jacks (HJ) have been adopted as follows:

- $\quad$ AM peak hour vehicle trips $=188(M C D)+0(K F C)+45(H J) \times 2=278$ vehicles per hour
- PM peak hour vehicle trips $=183(\mathrm{McD})+67(\mathrm{KFC})+63(\mathrm{HJ}) \times 2=376$ vehicles per hour
- SAT peak hour vehicle trips $=225(\mathrm{McD})+121(\mathrm{KFC})+110(\mathrm{HJ}) \times 2=566$ vehicles per hour.

The Roads and Maritime guide also indicates that approximately 50 per cent ${ }^{1}$ of the fast food traffic generation will be existing passing traffic on Pacific Highway. This does not discount the traffic generation rate for the use, but rather redirects existing traffic movements to the site to account for pass-by trips.

### 6.1.11 Petrol Station

The Roads and Maritime Guide provides a formula for the traffic generation of a petrol station based on the number of service channels (N) and total site area (S). On this basis, TTPP has assumed the petrol station will include around eight service channels. The petrol station has a total site area of $590 \mathrm{~m}^{2}$.

- AM peak hour vehicle trips $=0.2815\left(N^{2}\right)+14.047(N)+16.715=147$ vehicles per hour
- PM peak hour vehicle trips $=0.0205(S)+88.52=101$ vehicles per hour
- SAT peak hour vehicle trips $=$ No rate provided. PM rate assumed. $=101$ vehicles per hour.


### 6.1.12 Car Wash

The Roads and Maritime guide does not provide a trip rate for a car wash. A rate has instead been obtained from the "Proposed Carwash, 412-416 Liverpool Road, Croydon" report (K\&H Kane Investments Pty Ltd, 2006), which provides an average rate based on the survey data of several car wash facilities. On this basis, a rate of 13.3 trips per car wash lane has been adopted, for all three peak periods (AM, PM, SAT).

[^2]transport planning

### 6.1.13 Raw Challenge

Raw Challenge is an existing event that occurs twice a year and accommodates parking within the existing RSL Club site. The event would be subject to a separate Event Management Plan, with special traffic and parking management measures applied during the event e.g. the existing event includes temporary informal parking provision on the sporting fields. On this basis, traffic generation for Raw Challenge is not considered as part of the traffic generation estimate for the proposed development.

### 6.1.14 Go Kart

The Roads and Maritime guide does not provide a trip rate for facilities similar to Go Karting activities.

A review of the parking supply for an existing Go Kart and Paint Ball facility (Luddenham Raceway) has been undertaken to determine the traffic generation of the proposed site based on the car parking supply. It is assumed that the proposed Go Kart land use would require a car parking supply of 40 spaces, as per the provision at Luddenham Raceway (approximate). On this basis, a trip generation of 0.5 trips per car space per hour has been assumed i.e. 20 vehicle trips per hour.

Leisure and recreational trip rate will be reduced as some patrons will frequent the Club for drinks/ dinner after their event etc. i.e. linked trips.

### 6.1.15 Recreational Warehouse

Similarly, to the Go Kart land use, the recreational warehouse trip generation has been based on parking supply, with a rate of 0.5 trips per car space per hour. Based on existing recreational facilities (as discussed in Section 5.1), the recreational warehouse requires a parking supply of 117 spaces. On this basis, a trip generation of 59 vehicles per hour has been estimated during the road network peaks.

### 6.1.16 Senior Living Dwellings

The Roads and Maritime Guide states a rate of 0.4 trips per dwelling during the afternoon peak period for developments classed as "housing for seniors". The guide notes that the morning peak period of the site does not coincide with the road network peak period, however, for the purposes of this assessment, a rate of 0.4 trips per dwelling has been adopted for the weekday morning and Saturday peak period.

### 6.1.17 Residential

The Roads and Maritime Guide states the following traffic generation rates for low density residential dwellings in a regional area:

- $\quad$ AM peak hour vehicle trips $=0.78$ trips per dwelling
- PM peak hour vehicle trips $=0.71$ trips per dwelling
- $\quad$ SAT peak hour vehicle trips $=0.75$ trips per dwelling (the guide does not specify a rate for the Saturday peak hour, as such the average of the AM and PM trip rate has been assumed).


### 6.1.18 Traffic Generation Estimate

Based on the above traffic generation assumptions, the trip generation estimate for the proposed development has been calculated and is summarised in Table 6.2.

Table 6.2 indicates that the development generates a net increase of $1,387 \mathrm{vph}, 1,743 \mathrm{vph}$ and $1,844 \mathrm{vph}$ in the weekday morning, weekday afternoon and Saturday morning peak periods respectively.

Taking into consideration that approximately 50 per cent of the fast food and petrol station trade would include pass-by traffic on the Pacific Highway, the additional traffic generated on the road network is estimated as $1,056 \mathrm{vph}, 1,185 \mathrm{vph}$ and $1,488 \mathrm{vph}$ morning, afternoon and Saturday peak periods respectively.

## Table 6.2: $\quad$ Trip Generation Summary

| Land Use | Existing Size | Size (GFA) | Design Trip Rate (Road Network Peak) |  |  | Trip Rate Estimate (Road Network Peak) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | AM (7:30-8:30) | PM (15:30-16:30) | SAT (11:00-12:00) | AM | PM | SAT |
| RSL Club |  |  |  |  |  |  |  |  |
| - Club | 7,423.63 | 7,500 | 1.3 trips per 100sqm | 3.4 trips per 100sqm | 2.5 trips per 100sqm | 98 | 255 | 188 |
| - Dining Area/Food Court |  | 3,390 | 1.3 trips per 100sqm | 3.4 trips per 100sqm | 2.5 trips per 100sqm | 44 | 115 | 85 |
| - Function Room (750m²) |  | 500 people | NA | $83 \%$ car usage +2.2 car occupancy $+75 \%$ arrival rate $+50 \%$ overlap in events |  |  | NA | 71 |
| Hairdresser |  | 120 | 2.3 trips per 100sqm | 4.6 trips per 100sqm | 5.6 trips per 100sqm | 2.8 | 5.5 | 6.7 |
| Dry Cleaner |  | 120 | 2.3 trips per 100sqm | 4.6 trips per 100sqm | 5.6 trips per 100sqm | 2.8 | 5.5 | 6.7 |
| Subtotal |  |  |  |  |  | 147 | 452 | 356 |
| Health \& Wellness Precinct |  |  |  |  |  |  |  |  |
| - Gym | 1,679.01 | 3,000 | 3.2 trips per 100sqm | 4.6 trips per 100sqm | 2.2 trips per 100sqm | 96 | 138 | 66 |
| - Spa |  | 350 | 1.3 trips per 100sqm | 3.4 trips per 100sqm | 2.5 trips per 100sqm | 5 | 12 | 9 |
| - Swimming Pool |  | 16 lanes | 6.1 trips per lane | 6.1 trips per lane | 6.1 trips per lane | 98 | 98 | 98 |
| - Medical Clinic |  | 4,830 | 3.1 trips per 100sqm | 2.5 trips per 100sqm | 11 trips per 100sqm | 150 | 121 | 531 |
| - Physio |  | 150 | 3.1 trips per 100sqm | 2.5 trips per 100sqm | 11 trips per 100sqm | 5 | 4 | 17 |
| Subtotal |  |  |  |  |  | 353 | 372 | 720 |
| Motel/hotel ( $7,000 \mathrm{~m}^{2}-102$ units +98 villas) |  | 174 units | 0.4 per unit | 0.4 per unit | 0.4 per unit | 70 | 70 | 70 |
| Childcare ( $5,632 \mathrm{~m}^{2}$ ) |  | 360 places | 0.64 per child | 0.34 per child | NA | 230 | 122 | 0 |
| Fast Food Services |  |  |  |  |  |  |  |  |
| - Fast Food Outlet ( $4 \times 60$ seats ea) |  | 1,120 | $188+0+45+45$ | $183+67+63+63$ | $225+121+110+110$ | 278 | 376 | 566 |
| - Car Wash |  | 30 | 13.3 trips per bay | 13.3 trips per bay | 13.3 trips per bay | 13 | 13 | 13 |



### 6.2 Roads and Maritime Traffic Signal Warrants

Based on the Roads and Maritime Services Traffic Signal Design 2008 manual, a signalised intersection may be considered if warrants are met, either based on crash history, pedestrian safety, high speeds, or high traffic volumes.

Based on this, a traffic signal warrants assessment has been undertaken for the intersection of Pacific Highway and the proposed site access.

The relevant warrant has been reproduced and assessed within Table 6.3.
Table 6.3: Warrants Assessment for Proposed Site Access-Pacific Hwy
Warrants Met?
(Yes/No)
(a) Traffic demand: For each of four one-hour periods of an average day:

| (ii) the major road flow exceeds 600 vehicles/hour in each direction; and | Yes |
| :--- | :---: |
| (ii) the minor road flow exceeds 200 vehicles/hour in one direction. | Yes |
| Overall | Yes |

Pacific Highway is recorded to contain between 1,500 to 2,500 vehicles per hour during the survey periods (6:00-9:00, 15:30-19:00), with directional traffic ranging between 500 to 1,400 vehicles per hour in each direction. On this basis, the above flow warrants for a major road are satisfied (>600 vehicles per hour in each direction over four separate, one-hour periods).

The proposed site is anticipated to generate up to 1,395 vehicles per hour in the morning, 1,743 vehicles per hour in the afternoon on a weekday and 2,059 vehicles per hour on a Saturday. Assuming that the secondary access would generate some 80 vehicles per hour and Wentworth Avenue some 30-40 vehicles per hour, the primary access would generate around 1,300 to 1,900 vehicles per hour. This equates to over 650 vehicles per hour in any direction during the peak hour.

With the proposed land uses to operate and generate traffic throughout the day, the proposed site access will satisfy the traffic signal warrant for a minor road i.e. greater than 200 vehicles per hour in one direction for four one-hour periods.

### 6.3 Traffic Modelling

### 6.3.1 Scenarios

SIDRA intersection modelling has been carried out for nominated intersections. The following five scenarios have been assessed in this regard:
transport planning

- Scenario 1 - 2018 Existing
- Scenario 2-2018 Existing + Stage 1 Development
- Scenario 3-2028 Base (+10-year background growth)
- Scenario 4-2028 Base + Stage 1 Development
- Scenario 5 - 2038 Base (+20-year background growth)
- Scenario 6-2038 Base + Stage 2 Development

The intersection turning volumes for each of the above scenarios is provided in Appendix A.

### 6.3.2 Background Growth Factors

Background growth factors have been applied to general traffic on Pacific Highway, for the future scenarios using SIDRA.

Background traffic growth has been adopted based on Sydney Traffic Forecasting Model (STFM) growth plots obtained from Roads and Maritime Services. The STFM growth plots provide growth rates (per cent per annum growth) from the year 2016 to a 10-year future (2026) and a 20-year future (2036), along the Pacific Highway.

Based on a base assessment year of 2018, the 2016-2026 growth rates have been similarly adopted by TTPP for the 10-year post development scenario (2028) and the 2016-2036 rates for the 20-year post development scenario (2038). The STFM growth plots are attached in Appendix B for reference.

### 6.3.3 Traffic Distribution

Various factors impact the traffic distribution patterns of developments such as the location of employment and residential precincts, the layout of arterial road network, usage patterns of the subject land use, location of site access points etc.

In the case of the subject site, traffic has been distributed based upon existing traffic patterns on the road network. Notably, traffic survey data indicates that typically 75 per cent of traffic generated by the site travels in the southbound direction and 25 per cent in the northbound direction.

In relation to inbound/ outbound splits, the net additional two-way development traffic is expected to have the following splits based on survey data and industry typical assumptions:

- Existing uses (RSL Club, Gym) as per the existing distribution - $70 \%$ inbound/ $30 \%$ outbound in AM peak, $60 \%$ inbound/ $40 \%$ outbound in PM peak, $65 \%$ inbound/ $35 \%$ outbound in SAT peak
- Function Room - $77 \%$ inbound/ $23 \%$ outbound in PM peak (as discussed in Section 6.1.2), $50 \%$ inbound/ $50 \%$ outbound in SAT peak
- Health \& Wellness Precinct $-50 \%$ inbound/ $50 \%$ outbound
- Motel/ Hotel - $50 \%$ inbound/ $50 \%$ outbound
- Childcare - $50 \%$ inbound/ $50 \%$ outbound
- Retail/ Fast Food - $50 \%$ inbound/ $50 \%$ outbound
- Go Kart/Recreational warehouse - 50\% inbound/ 50\% outbound
- Residential/ Senior Living - $20 \%$ inbound/ $80 \%$ outbound in the AM Peak, $80 \%$ inbound/ $20 \%$ outbound in the PM Peak.

The recreational component of the site has a separate access point with left-in/left-out movements only permitted. On this basis, it is assumed that outbound recreational traffic would turn left at the new signalised intersection and undertake a u-turn movement at the internal roundabout to travel northbound and inbound recreational traffic from the south would undertake a u-turn manoeuvre at the u-turn facility located approximately 60-metres north of the recreational site access, and then turn left into the site.

In addition, a portion of the residential and senior living traffic has been distributed to the site via Wentworth Avenue. However, the intersection modelling detailed in Section 6.4 indicates that the intersection of Wentworth Avenue and Pacific Highway will be at capacity in the future due to delays associated with traffic from Wentworth Avenue giving way to increasing through traffic along Pacific Highway. On this basis, the volume of development traffic anticipated to use Wentworth Avenue is anticipated to be nominal, with the proposed signalised intersection anticipated to provide a better access point to the site. For the purposes of this assessment, 25 per cent of residential/senior living traffic is assumed to use Wentworth Avenue, with the exception of those turning right into Pacific Highway, who are likely to use the new signalised intersection to travel northbound rather than give way to two directions of traffic.

### 6.4 Intersection Modelling

### 6.4.1 Intersection Modelling Criteria

The key intersections surrounding the site have been assessed using SIDRA Intersection 8, a computer-based modelling package which assesses intersection performance under prevailing traffic conditions.

SIDRA calculates intersection performance measures such as 'average delay' that vehicles encounter and the level of service (LOS). SIDRA provides analysis of the operating conditions which can be compared to the performance criteria set out in Table 6.4.

Table 6.4: Level of Service Criteria for Intersection Operation

| Level of Service | Average Delay (seconds per vehicle) | Traffic Signals, Roundabout | Give Way and Stop Signs |
| :---: | :---: | :---: | :---: |
| A | Less than 14 | good operation | good operation |
| B | 15 to 28 | good with acceptable delays and spare capacity | acceptable delays and spare capacity |
| C | 29 to 42 | satisfactory | satisfactory, but accident study required |
| D | 43 to 56 | operating near capacity | near capacity and accident study required |
| E | 57 to 70 | at capacity <br> At signals, incidents will cause excessive delays. | at capacity, requires other control mode |
| F | Greater than 71 | unsatisfactory with excessive queuing | unsatisfactory with excessive queuing; requires other control mode |

Source: Roads and Maritime Guide to Traffic Generating Developments, 2002

### 6.4.2 Scenario 1 - 2018 Existing

The existing operation of the key study intersections is summarised in Table 6.5.
Table 6.5: Scenario 1-2018 Existing Intersection Operation

| Leg | Intersection Control | AM Peak |  | PM Peak |  | SAT Peak |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Ave. Delay (sec/veh) | Level of Service | Ave. <br> Delay (sec/veh) | Level of Service | Ave. Delay (sec/veh) | Level of Service |
| Pacific Hwy - Wyee Rd - Scenic Dr | Signalised | 41 | C | 73 | F | 45 | D |
| Pacific Hwy Wentworth Ave | Priority | 46 | D [1] | 25 | B | 27 | B |
| Proposed Site Access | NA |  |  |  |  |  |  |
| Ruttleys Road - Pacific Highway | Signalised | 13 | A | 17 | B | 14 | A |

[1] Related to the right-turn movement from Wentworth Avenue which includes 12 vehicles per hour under existing conditions and an estimated 4 vehicles per hour under post development conditions in the morning peak

Table 6.5 indicates the following key intersection results:

- Pacific Highway-Wyee Road-Scenic Drive is at capacity during the weekday afternoon peak with a LOS F and operates acceptably with a LOS C in the weekday morning peak and is nearing capacity with a LoS D during the Saturday peak period.
- Pacific Highway-Wentworth Avenue is near capacity in the weekday morning peak with a LOS D (relating to the right turn movement from Wentworth Avenue) and operates well in the weekday afternoon and Saturday peaks.
- Ruttleys Road-Pacific Highway operates well with a LoS B or better.


### 6.4.3 Scenario 2 - 2018 Existing + Stage 1 Development

Sidra modelling was undertaken to assess the impact of the Stage 1 development on the road network under existing conditions. The results of the assessment are summarised in Table 6.6.

Table 6.6: Scenario 2-2018 Existing + Stage 1 Intersection Operation

| Leg |  | AM Peak |  | PM Peak |  | SAT Peak |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Intersection <br> Control | Ave. <br> Delay <br> (sec/veh) | Level of <br> Service | Ave. <br> Delay <br> (sec/veh) | Level of <br> Service | Ave. <br> Delay <br> (sec/veh) | Level of <br> Service |
| Pacific Hwy - Wyee Rd <br> - Scenic Dr |  | 47 | D | 125 | F | 54 | D |
| Pacific Hwy - <br> Wentworth Ave | Priority | 52 | D [1] | 26 | B | 27 | B |
| Proposed Site Access | Signalised | 16 | B | 15 | B | 16 | B |
| Ruttleys Road - Pacific <br> Highway | Signalised | 13 | A | 17 | B | 15 | A |

[1] Related to the right-turn movement from Wentworth Avenue which includes 12 vehicles per hour under existing conditions and an estimated 4 vehicles per hour under post development conditions in the morning peak

Based on Table 6.6, additional development traffic will increase delays to the existing intersections of Pacific Highway-Wyee Road-Scenic Drive and Pacific Highway-Wentworth Avenue, which already operate near or above capacity.

Wentworth Avenue with its current configuration, would continue to operate at a LoS D with a minor increase in delay to existing turning movements to and from the Pacific Highway. A proposed new site access on Pacific Highway would reduce the number of traffic movements at this intersection, however overall delays are anticipated to increase, due to the associated increase in through traffic on the Pacific Highway. However, under existing conditions there are only 12 vehicles per hour undertaking this right furn from Wentworth Avenue in the morning peak. Following the relocation of the RSL Club, this is anticipated to reduce further (estimated as 4 vehicles per hour from residential traffic). On this basis, the volume of traffic impacted by an increase to delay would be minimal.

The intersection of Ruttleys Road-Pacific Highway would continue to operate well.

### 6.4.4 Scenario 3 - 2028 Base (Do Nothing. Without development)

An assessment of the study intersections during Year 2028 without any changes to the subject RSL Club has been undertaken. The results of the 2028 base year are summarised in Table 6.7.

Table 6.7: Scenario 3-2028 Base Intersection Operation

| Leg | Intersection Control | AM Peak |  | PM Peak |  | SAT Peak |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Ave. Delay (sec/veh) | Level of Service | Ave. Delay (sec/veh) | Level of Service | Ave. Delay (sec/veh) | Level of Service |
| Pacific Hwy - Wyee Rd - Scenic Dr | Signalised | 71 | F | 126 | F | 64 | E |
| Pacific Hwy Wentworth Ave | Priority | 65 | E | 30 | C | 33 | C |
| Proposed Site Access | NA |  |  |  |  |  |  |
| Ruttleys Road - Pacific Highway | Signalised | 13 | A | 22 | B | 16 | B |

Table 6.7 indicates that the intersection of Pacific Highway-Wyee Road-Scenic Drive and Pacific Highway-Wentworth Avenue would operate at capacity with estimated background traffic growth by year 2028.

The intersection of Ruttleys Road-Pacific Highway would continue to operate well with spare capacity.

### 6.4.5 Scenario 4-2028 + Stage 1 Development

Further assessment of the study intersections during the year 2028 with the proposed Stage 1 development has been undertaken. The results of the 2028 base year plus the Stage 1 development model are summarised in Table 6.8.

Table 6.8: Scenario 4-2028 + Stage 1 Development - Intersection Operation

| Leg | Intersection Control | AM Peak |  | PM Peak |  | SAT Peak |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Ave. Delay (sec/veh) | Level of Service | Ave. Delay (sec/veh) | Level of Service | Ave. Delay (sec/veh) | Level of Service |
| Pacific Hwy - Wyee Rd - Scenic Dr | Signalised | 86 | F | 205 | F | 80 | F |
| Pacific Hwy Wentworth Ave | Priority | 83 | F [1] | 38 | C | 36 | C |
| Proposed Site Access | Signalised | 16 | B | 15 | B | 16 | B |
| Ruttleys Road - Pacific Highway | Signalised | 14 | A | 24 | B | 16 | B |

[1] Related to the right-turn movement from Wentworth Avenue which includes 12 vehicles per hour under existing conditions and an estimated 4 vehicles per hour under post development conditions in the morning peak

Table 6.8 indicates that the proposed Stage 1 development would increase delays to the study intersections of Pacific Highway-Wyee Road-Scenic Drive and Pacific HighwayWentworth Avenue, which are already at capacity by 2028.

Notably, the intersection of Wentworth Avenue and Pacific Highway will operate at a LoS F, where under Year 2028 Base conditions, the intersection operates as a LoS E. This indicates that while the turning traffic volumes at Wentworth Avenue would reduce as a result of the proposed development, the overall delay to vehicles entering and exiting Wentworth Avenue would increase, due to an associated increase in through traffic along Pacific Highway. In short, turning vehicles would be required to give-way to additional through traffic. However, it is noted that the LoS F relates to the right turn movement from Wentworth Avenue to Pacific Highway only. Motorists turning right experience greater delay as they are required to give-way to two directions of traffic. On the other hand, the left turning movement from Wentworth Avenue into Pacific Highway is anticipated to operate satisfactorily with a LoS B.

Notwithstanding the impact to the right turn movement, it is considered that the intersection of Wentworth Avenue and Pacific Highway would operate more safely as the volume of turning movements at this intersection will reduce considerably.

The intersection of Ruttley Road and Pacific Highway and the proposed site access would continue to operate well under post development conditions in 2028.

### 6.4.6 Scenario 5 - 2038 Base (Do Nothing. Without Development)

An assessment of the study intersections 20 years in the future (year 2038) without any changes to the subject RSL Club has been undertaken. The results of the assessment are summarised in Table 6.9.

Table 6.9: Scenario 5-2038 Base Intersection Operation

| Leg | Intersection Control | AM Peak |  | PM Peak |  | SAT Peak |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Ave. <br> Delay (sec/veh) | Level of Service | Ave. <br> Delay (sec/veh) | Level of Service | Ave. Delay (sec/veh) | Level of Service |
| Pacific Hwy - Wyee Rd - Scenic Dr | Signalised | 115 | F | 213 | F | 96 | F |
| Pacific Hwy Wentworth Ave | Priority | 104 | F | 45 | D | 51 | D |
| Proposed Site Access | NA |  |  |  |  |  |  |
| Ruttleys Road - Pacific Highway | Signalised | 15 | B | 34 | C | 17 | B |

Table 6.9 indicates that estimated background traffic growth to year 2038 would significantly increase delays to the intersections of Pacific Highway-Wyee Road-Scenic Drive and Pacific Highway-Wentworth Avenue which are already operating at or near capacity. Notably, Pacific Highway-Wentworth Avenue would operate at a LOS D during the afternoon and weekend peak where under existing and year 2028 scenarios the intersection was operating at a LoS C. The intersection is already at capacity under existing and forecasted Year 2028 conditions. By Year 2038 the intersection is anticipated to well exceed capacity.

There would also be an increase in delay to the intersection of Ruttleys Road-Pacific Highway, however acceptable levels of intersection performance would be maintained i.e. LOS C or better.

### 6.4.7 Scenario 6-2038 + Stage 2 Development

Year 2038 was reassessed with the full inclusion of the development site (Stage 1 and Stage 2 development). The assessment results are summarised in Table 6.10.

Table 6.10: Scenario 6-2038 + Stage 2 Intersection Operation

| Leg | Intersection Control | AM Peak |  | PM Peak |  | SAT Peak |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Ave. Delay (sec/veh) | Level of Service | Ave. Delay (sec/veh) | Level of Service | Ave. Delay (sec/veh) | Level of Service |
| Pacific Hwy - Wyee Rd - Scenic Dr | Signalised | 189 | F | 372 | F | 231 | F |
| Pacific Hwy Wentworth Ave | Priority | 180 | F | 71 | F | 134 | F |
| Proposed Site Access | Signalised | 20 | B | 19 | B | 25 | B |
| Ruttleys Road - Pacific Highway | Signalised | 15 | B | 27 | B | 20 | B |

As with the 2038 base scenario (without development), the intersections of Pacific HighwayWyee Road-Scenic Drive and Pacific Highway-Wentworth Avenue would continue to be over capacity with excessive delays.

There would also be an increase in delay to the intersection of Ruttleys Road-Pacific Highway intersection, however, acceptable levels of intersection performance (LOS B) could be maintained by the dynamic reallocation of green time to the Pacific Highway approaches.

The proposed site access from Pacific Highway is anticipated to operate satisfactorily with a LoS B in year 2038 with inclusion of the full Doyalson Wyee RSL Structure Plan site.

Based on the above, it is considered that residents would instinctively use the new signalised access in place of the Wentworth Avenue and Pacific Highway intersection, which would be at capacity by the year 2038. The proposed site access is indicated to have substantial spare capacity by the year 2038 and would be able accommodate traffic redirected from Wentworth Avenue.

### 6.4.8 Intersection Assessment Summary

The SIDRA intersection assessment as detailed above indicates the following key findings:

- The Pacific Highway, Wyee Road and Scenic Drive intersection is currently operating over capacity during the afternoon peak period. The addition of general background traffic
growth and the proposed development traffic would put further pressure on the intersection.
- The Pacific Highway and Wentworth Avenue intersection is nearing capacity due to the heavy traffic flows along the Pacific Highway delaying turning movements from Wentworth Avenue, particularly the right turn movement in the morning peak period. However, the right turning volume is relatively low with 12 vehicles per hour and with the relocation of the RSL Club, traffic volumes along Wentworth Avenue are anticipated to reduce further. On this basis, the volume of traffic impacted by an increase in Pacific Highway through traffic would be minimal.
- Furthermore, with a reduction in turning traffic movements, the relocation of the RSL Club is anticipated to improve safety and reduce the occurrence of accidents at the Pacific Highway and Wentworth Avenue intersection.
- The residential traffic that is proposed as part of the development is anticipated to largely access the site via the new signalised intersection, to avoid delays at the Wentworth Avenue and Pacific Highway intersection.
- The proposed signalised site access has been designed to operate at a LoS B following the completion of the full development and would have spare capacity to accommodate any additional detoured residential traffic.
- The intersection of Ruttleys Road and Pacific Highway currently operates well with spare capacity. The intersection is anticipated to continue operating satisfactorily in year 2038 following completion of the full Doyalson Structure Plan. It is noted that some minor relocation of green time would be required, however no adverse impacts are anticipated from this relocation of green time.

On this basis, the operation of the intersection of Pacific Highway, Wyee Road and Scenic Drive is required to be upgraded to accommodate future traffic from both general background traffic growth and the subject Structure Plan. However, as discussed in Section 3.7.3, a detailed assessment is currently being undertaken by Roads and Maritime Services, with an aim to upgrade the intersection and accommodate future growth in traffic.

Detailed SIDRA results are provided in Appendix C.

### 6.5 Intersection Design

Intersection modelling has been undertaken for a new signalised intersection with Pacific Highway and a proposed access into the site. The concept layout of the intersection is provided in Appendix D.

The concept design of the proposed signalised site access has been based on Austroads guidance for road design. Some key requirements are as follows.

### 6.5.1 Deceleration Lane Lengths

The required deceleration lane lengths for turning lanes on the Pacific Highway has been assessed, based on Austroads (2017). The required length for a deceleration lane in Table 6.11 .

Table 6.11: Deceleration distances required for cars on a level grade

| Design speed of approach road (km/h) | Length of deceleration $\mathrm{D}-$ including diverge taper $T(\mathrm{~m})$ |  |  |  |  |  |  |  |  |  | Diverge length $L_{d}{ }^{(3)}$ for lane widths ( m ) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Stop condition ${ }^{(1)}$ (m) |  | Design speed of exit curve (km/h) ${ }^{(2]}$ |  |  |  |  |  |  |  |  |  |
|  | 0 | 0 | 20 | 30 | 40 | 50 | 60 | 70 | 80 | 90 | $3.5 \mathrm{~m}^{(4)}$ | $3.0 \mathrm{~m}^{(4)}$ |
|  | Comfortable $2.5 \mathrm{~m} / \mathrm{s}^{2}$ | Maximum $3.5 \mathrm{~m} / \mathrm{s}^{2}$ | Comfortable average rate of deceleration $2.5 \mathrm{~m} / \mathrm{s}^{2}$ |  |  |  |  |  |  |  |  |  |
| 50 | 40 | 30 | 30 | 25 | 15 |  |  |  |  |  | 33 | 27 |
| 60 | 55 | 40 | 50 | 40 | 30 | 15 |  |  |  |  | 40 | 33 |
| 70 | 75 | 55 | 70 | 60 | 50 | 40 | 20 |  |  |  | 47 | 40 |
| 80 | (100) | 70 | 95 | 85 | 75 | 60 | 45 | 25 |  |  | 54 | 44 |
| 90 | 125 | 90 | 120 | 110 | 100 | 85 | 70 | 50 | 25 |  | 60 | 50 |
| 100 | 155 | 110 | 150 | 140 | 130 | 115 | 100 | 80 | 55 | 30 | 67 | 57 |
| 110 | 185 | 135 | 180 | 175 | 160 | 150 | 130 | 110 | 90 | 60 | 74 | 62 |

$\begin{array}{ll}1 & \text { Rates of deceleration are: } 2.5 \mathrm{~m} / \mathrm{s}^{2} \text { for comfortable deceleration; } 3.5 \mathrm{~m} / \mathrm{s}^{2} \text { is the maximum for design purposes. } \\ 2 & \text { Speed of exit curve depends on radius and crossfall (Figure } 5.2 \text { ). } \\ 3 & \text { Distance Ld assumes a lateral rate of movement of } 1.5 \mathrm{~m} / \mathrm{s} . \\ 4 & \text { Example lane widths - use actual lateral shift distance of vehicle. }\end{array}$

Based on the above, vehicles coming to a stop on Pacific Highway (to turn right into the site or to stop at signals), a deceleration lane length of 100 m is required. Where vehicles are decelerating to turn (i.e. left turn treatment), a minimum 95-metre long deceleration lane is required.

On this basis, a 95m deceleration lane has been provided for the left turn from Pacific Highway into the site and a 100 m deceleration lane for the right turn into the site, in addition to the storage capacity required to hold estimated vehicle queues (based on the SIDRA assessment).

### 6.5.2 Sight Distance Requirements

The sight distance requirements for the proposed access road is defined in the Austroads (2017) Guide to Road Design Part 4A.

Stopping Sight Distance (SSD) is the sight distance required to enable a driver to observe, react and stop safely. On this basis, on approach to intersections where drivers are required to stop, an appropriate SSD must be ensured.

The SSD can be calculated using Table 6.12, which indicates that for an $80 \mathrm{~km} / \mathrm{h}$ road (i.e. Pacific Highway), a minimum SSD of 114 metres is required.

Table 6.12: $\quad$ Stopping Sight Distance

| Design speed (km/h) | Absolute minimum values Only for specific road types and situations ${ }^{(1)}$ based on $d=0.46^{(2),(3)}$ |  |  | Desirable minimum values for all road types based on $d=0.36$ |  |  | Values for major highways and freeways in flat terrain ${ }^{(7)}$ <br> based on $d=0.26$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $R_{T}=1.5 \mathrm{~s}^{(4)}$ | $\mathrm{R}_{\mathrm{T}}=2.0 \mathrm{~s}^{(4)}$ | $R_{T}=2.5 \mathrm{~s}$ | $\mathrm{R}_{\mathrm{T}}=1.5 \mathrm{~s}^{(4)}$ | $\mathrm{R}_{\mathrm{T}}=2.0 \mathrm{~s}^{(4)}$ | $\mathrm{R}_{\mathrm{T}}=2.5 \mathrm{~s}$ | $R_{T}=2.0 \mathrm{~s}$ | $R_{T}=2.5 \mathrm{~s}$ |
| 40 | 30 | 36 | - | 34 | 40 | 45 | - | - |
| 50 | 42 | 49 | - | 48 | 55 | 62 | - | - |
| 60 | 56 | 64 | - | 64 | 73 | 81 | - | - |
| 70 | 71 | 81 | - | 83 | 92 | 102 | 113 | 123 |
| 80 | 88 | 99 | - | 103 | 114 | 126 | 141 | 152 |
| 90 | 107 | 119 | 132 | 126 | 139 | 151 | 173 | 185 |
| 100 | - | 141 | 155 | - | 165 | 179 | 207 | 221 |
| 110 | - | 165 | 180 | - | 193 | 209 | 244 | 260 |
| 120 | - | 190 | 207 | - | 224 | 241 | 285 | 301 |
| 130 | - | 217 | 235 | - | 257 | 275 | 328 | 346 |

Source: Austroads, 2016, Guide to Road Design Part 3: Geometric Design
The existing sight distances along Pacific Highway from the location of the proposed site access includes approximately 400 m looking to the north and approximately 380 m looking to the south. Therefore, the proposed intersection would meet the Austroads SSD requirement of 114 m . Site photos along Pacific Highway from the proposed access location are shown in Figure 6.1 and Figure 6.2.

Figure 6.1: Looking north from access


Figure 6.2: Looking south from access

transport planning

## 7 Conclusion

Doyalson-Wyee RSL Limited is proposing to amalgamate several properties to develop a mixed-use site, to diversify the services and facilities of the RSL. The Structure Plan for the future site includes a combination of land uses in addition to the existing RSL Club including: residential, recreational, fast food, childcare, motel/hotel, health and fitness, medical and retail uses.

A traffic and transport assessment has been undertaken to assess the impacts of the Structure Plan. The key findings of the assessment are as follows:

- A Planning Proposal is to be submitted for a 20-year Structure Plan for the Doyalson Wyee RSL Club. For the purposes of the traffic and transport assessment, the development has been indicatively split into two key stages which includes:
- Stage 1 (10-year structure plan) which includes the Stage 1 RSL Club and function room, partial senior living housing, fast services (fast food outlets, car wash, petrol station), childcare centre, hotel and a portion of the Health and Wellness Precinct (Gym)
- Stage 2 (20-year structure plan) which includes the Stage 2 RSL Club and function room, remaining senior living, residential, remaining Health and Wellness Precinct (spa, swimming pool, medical clinic, physio) and recreational uses (Go Kart, recreational warehouse).
- Based on an assessment of DCP parking requirements and cumulative parking demand for each land use, the commercial and recreational land uses will generate a peak parking demand of 1,108 spaces.
- A parking provision of 1,295 spaces is proposed to accommodate the development. In addition, an overflow car park for events is proposed accommodating an additional 104 spaces.
- The proposed development is anticipated to generate a net increase of $1,056 \mathrm{vph}$, $1,185 \mathrm{vph}$ and $1,488 \mathrm{vph}$ to the road network in the morning, afternoon and Saturday peak periods.
- SIDRA intersection modelling of key intersections indicates the following:
- The Pacific Highway, Wyee Road and Scenic Drive intersection is currently operating at capacity during the afternoon peak period. The addition of general background traffic growth and the proposed development traffic would put further pressure on the intersection.
- The Pacific Highway and Wentworth Avenue intersection is nearing capacity due to the heavy traffic flows along the Pacific Highway delaying turning movements from Wentworth Avenue, particularly the right turn movement in the morning peak period. However, the right turning volume is relatively low with 12 vehicles per hour and with the relocation of the RSL Club, traffic volumes along Wentworth Avenue are
anticipated to reduce further. On this basis, the volume of traffic impacted by an increase in Pacific Highway through traffic would be minimal.
- Furthermore, the relocation of the RSL Club and associated reduction in turning traffic movements at Wentworth Avenue would improve safety and reduce the occurrence of accidents at the Pacific Highway and Wentworth Avenue intersection.
- The intersection of Ruttleys Road and Pacific Highway currently operates well with spare capacity. The intersection is anticipated to continue operating satisfactorily to year 2038 following completion of the full Doyalson Structure Plan.
- The residential traffic that is proposed as part of the development is anticipated to largely access the site via the new signalised intersection, to avoid delays at the Wentworth Avenue and Pacific Highway intersection.
- The proposed signalised access has been designed to operate at a LoS B following the completion of the full development and would have spare capacity to accommodate any additional detoured residential traffic.
- Consultation with authorities has indicated two key strategic studies relevant to the development site:
- The Lake Munmorah Structure Plan is being prepared to assess the cumulative impact of several development proposals to occur within Doyalson North and Lake Munmorah. The Structure Plan will include an assessment of the impact to the Pacific Highway.
- Roads and Maritime Services are investigating the operation of the Pacific Highway, Wyee Road and Scenic Drive intersection, with an aim to improve the operation of the intersection to accommodate future growth in traffic. On this basis, TTPP have not assessed mitigation strategies to improve the operation of this intersection.
- A concept design of the proposed signalised site access has been prepared with consideration for SIDRA modelling results and Austroads guidelines on road design.
transport planning


## Appendix A

Traffic Movement Volumes

2018 EXISTING ROAD NETWORK PEAK VOLUMES
10 (10) [10] = AM (PM) [SAT] PEAKS
AM PEAK $=7: 30-8: 30$
MM PEAK = 16:45-17:45
SAT PEAK = 11:00-12:00



2018 EXISTING + STAGE 1 DEVELOPMENT ROAD NETWORK PEAK VOLUMES
10 (10) [10] = AM (PM) [SAT] PEAKS
AM PEAK $=7: 30-8: 30$
PM PEAK = 16:45-17:45
SAT PEAK = 11:00-12:00



2028 BASE (+ 10 YEAR BACKGROUND GROWTH) ROAD NETWORK PEAK VOLUMES
10 (10) [10] = AM (PM) [SAT] PEAKS
AM PEAK $=7: 30-8: 30$
PM PEAK = 16:45-17:45
SAT PEAK = 11:00-12:00




| rev. | DESCRIPTTON | DRamN | CHECK | APPD | DATE | ttpp transport planning | RoEET | DOYALSON WYEE RSL | DWG No.17395CAD005FIGURE 4 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | ISSUE F For discussion | kM | ${ }^{\text {of }}$ | w | 18/12/18 |  |  |  |  |  |  |
|  |  |  |  |  |  |  | TTTLE |  | ${ }^{\text {Date STAMP }} 18$ DECEMBER 2018 |  |  |
|  |  |  |  |  |  |  |  | NETWORK MOVEMENT DIAGRAM | ProJECT No. <br> 17395 | \|lale | A |

2038 BASE (+20-YEAR BACKGROUND GROWTH ) ROAD NETWORK PEAK VOLUMES
10 (10) [10] = AM (PM) [SAT] PEAKS
AM PEAK $=7: 30-8: 30$
PM PEAK $=16: 45-17: 45$
SAT PEAK = 11:00-12:00



AM PEAK = 7:30-8:30
PM PEAK = 16:45-17:45
SAT PEAK = 11:00-12:00


transport planning

## Appendix B

## STFM Growth Plots






## Appendix C

SIDRA Outputs

## MOVEMENT SUMMARY

## Site: 101 [Pacific HIghway-Wyee Road-Scenic Drive -AM Ex (7:30-8:30)]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 104 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Demand <br> Total veh/h | $\begin{array}{r} \text { Flows } \\ \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 463 | 1.6 | 0.252 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.9 |
| 22 | T1 | 155 | 8.2 | 0.884 | 58.2 | LOS E | 8.8 | 65.6 | 0.97 | 1.00 | 1.45 | 30.9 |
| 23 | R2 | 181 | 4.1 | 0.663 | 52.3 | LOS D | 9.0 | 65.4 | 0.99 | 0.83 | 1.03 | 28.0 |
| Appr |  | 799 | 3.4 | 0.884 | 26.4 | LOS B | 9.0 | 65.6 | 0.41 | 0.69 | 0.52 | 41.2 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 92 | 5.7 | 0.101 | 14.3 | LOS A | 1.4 | 9.9 | 0.54 | 0.72 | 0.54 | 47.1 |
| 25 | T1 | 1254 | 3.2 | 0.949 | 60.9 | LOS E | 41.8 | 300.9 | 1.00 | 1.14 | 1.38 | 28.7 |
| 26 | R2 | 34 | 0.0 | 0.377 | 62.9 | LOS E | 1.8 | 12.6 | 1.00 | 0.72 | 1.00 | 25.6 |
| Appr |  | 1379 | 3.3 | 0.949 | 57.8 | LOS E | 41.8 | 300.9 | 0.97 | 1.10 | 1.32 | 29.4 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 42 | 5.0 | 0.557 | 48.1 | LOS D | 7.3 | 54.2 | 0.97 | 0.83 | 1.21 | 30.8 |
| 28 | T1 | 122 | 7.8 | 0.557 | 42.5 | LOS C | 7.3 | 54.2 | 0.97 | 0.83 | 1.21 | 35.0 |
| 29 | R2 | 179 | 4.1 | 0.648 | 52.0 | LOS D | 8.9 | 64.4 | 0.99 | 0.83 | 1.02 | 33.5 |
| Appr |  | 343 | 5.5 | 0.648 | 48.1 | LOS D | 8.9 | 64.4 | 0.98 | 0.83 | 1.11 | 33.7 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 99 | 14.9 | 0.059 | 7.8 | LOS A | 0.0 | 0.0 | 0.00 | 0.59 | 0.00 | 61.8 |
| 31 | T1 | 745 | 9.5 | 0.469 | 22.5 | LOS B | 13.4 | 101.3 | 0.76 | 0.66 | 0.76 | 48.2 |
| 32 | R2 | 169 | 8.7 | 0.672 | 55.2 | LOS D | 8.6 | 64.5 | 1.00 | 0.83 | 1.06 | 33.4 |
| Approach |  | 1014 | 9.9 | 0.672 | 26.5 | LOS B | 13.4 | 101.3 | 0.73 | 0.68 | 0.74 | 45.2 |
| All V | icles | 3535 | 5.4 | 0.949 | 40.8 | LOS C | 41.8 | 300.9 | 0.78 | 0.86 | 0.95 | 36.1 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^3]
## MOVEMENT SUMMARY

## Site: 101 [Pacific HIghway-Wyee Road-Scenic Drive -PM Ex (4:45-5:45) ]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 184 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 232 | 3.2 | 0.128 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.8 |
| 22 | T1 | 155 | 2.0 | 0.880 | 90.7 | LOS F | 14.3 | 101.7 | 0.95 | 0.94 | 1.25 | 24.3 |
| 23 | R2 | 181 | 2.3 | 0.803 | 87.4 | LOS F | 15.9 | 113.2 | 0.98 | 0.87 | 1.11 | 20.6 |
| Appro |  | 567 | 2.6 | 0.880 | 54.9 | LOS D | 15.9 | 113.2 | 0.57 | 0.75 | 0.69 | 30.5 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 185 | 2.3 | 0.245 | 27.3 | LOS B | 6.9 | 49.0 | 0.69 | 0.77 | 0.69 | 38.6 |
| 25 | T1 | 769 | 4.7 | 0.917 | 88.8 | LOS F | 39.4 | 286.5 | 1.00 | 1.01 | 1.21 | 22.2 |
| 26 | R2 | 53 | 0.0 | 0.326 | 93.6 | LOS F | 4.5 | 31.7 | 0.98 | 0.76 | 0.98 | 19.9 |
| Appr |  | 1007 | 4.0 | 0.917 | 77.7 | LOS F | 39.4 | 286.5 | 0.94 | 0.95 | 1.10 | 23.9 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 35 | 3.0 | 0.726 | 99.6 | LOS F | 19.5 | 137.8 | 1.00 | 0.90 | 1.49 | 19.7 |
| 28 | T1 | 195 | 1.1 | 0.726 | 94.0 | LOS F | 19.5 | 137.8 | 1.00 | 0.90 | 1.49 | 23.6 |
| 29 | R2 | 166 | 0.6 | 0.843 | 91.8 | LOS F | 15.0 | 105.4 | 0.97 | 0.90 | 1.18 | 24.7 |
| Appr |  | 396 | 1.1 | 0.843 | 93.5 | LOS F | 19.5 | 137.8 | 0.99 | 0.90 | 1.36 | 23.8 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 121 | 2.6 | 0.066 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 65.4 |
| 31 | T1 | 1284 | 2.0 | 0.932 | 66.2 | LOS E | 69.9 | 497.8 | 0.91 | 0.95 | 1.06 | 27.2 |
| 32 | R2 | 467 | 1.1 | 1.013 | 104.1 | LOS F | 40.0 | 282.8 | 1.00 | 1.03 | 1.43 | 18.8 |
| Approach |  | 1873 | 1.9 | 1.013 | 71.9 | LOS F | 69.9 | 497.8 | 0.88 | 0.94 | 1.09 | 25.0 |
| All Ve | icles | 3843 | 2.4 | 1.013 | 73.1 | LOS F | 69.9 | 497.8 | 0.86 | 0.91 | 1.06 | 25.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^4]
## MOVEMENT SUMMARY

## Site: 101 [Pacific HIghway-Wyee Road-Scenic Drive -Sat Ex (11:00-12:00)]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 157 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 309 | 1.0 | 0.168 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.9 |
| 22 | T1 | 137 | 3.1 | 0.827 | 72.2 | LOS F | 10.3 | 74.3 | 0.94 | 0.88 | 1.19 | 27.6 |
| 23 | R2 | 191 | 2.2 | 0.764 | 74.0 | LOS F | 14.1 | 100.7 | 0.98 | 0.86 | 1.07 | 22.9 |
| Appro |  | 637 | 1.8 | 0.827 | 40.4 | LOS C | 14.1 | 100.7 | 0.49 | 0.70 | 0.58 | 35.1 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 155 | 1.4 | 0.186 | 21.3 | LOS B | 4.2 | 30.0 | 0.63 | 0.75 | 0.63 | 42.1 |
| 25 | T1 | 856 | 2.8 | 0.850 | 62.7 | LOS E | 33.4 | 239.2 | 0.99 | 0.94 | 1.09 | 28.2 |
| 26 | R2 | 41 | 5.1 | 0.514 | 91.1 | LOS F | 3.3 | 23.9 | 1.00 | 0.74 | 1.00 | 20.2 |
| Appr |  | 1052 | 2.7 | 0.850 | 57.7 | LOS E | 33.4 | 239.2 | 0.94 | 0.90 | 1.02 | 29.1 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 57 | 3.7 | 0.835 | 82.7 | LOS F | 14.3 | 102.9 | 1.00 | 0.97 | 1.44 | 22.2 |
| 28 | T1 | 129 | 3.3 | 0.835 | 77.0 | LOS F | 14.3 | 102.9 | 1.00 | 0.97 | 1.44 | 26.3 |
| 29 | R2 | 139 | 2.3 | 0.763 | 80.9 | LOS F | 10.7 | 76.3 | 1.00 | 0.86 | 1.13 | 26.6 |
| Appr |  | 325 | 2.9 | 0.835 | 79.7 | LOS F | 14.3 | 102.9 | 1.00 | 0.92 | 1.31 | 25.8 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 115 | 5.5 | 0.064 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 64.5 |
| 31 | T1 | 883 | 1.9 | 0.450 | 25.8 | LOS B | 21.1 | 149.8 | 0.68 | 0.61 | 0.68 | 45.5 |
| 32 | R2 | 304 | 1.4 | 0.702 | 37.8 | LOS C | 11.6 | 82.2 | 0.97 | 0.84 | 0.97 | 39.7 |
| Approach |  | 1302 | 2.1 | 0.702 | 27.0 | LOS B | 21.1 | 149.8 | 0.69 | 0.66 | 0.69 | 45.1 |
| All Vehicles |  | 3316 | 2.3 | 0.850 | 44.5 | LOS D | 33.4 | 239.2 | 0.76 | 0.77 | 0.83 | 34.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^5]
## MOVEMENT SUMMARY

ज्ञाTF Site: 1 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road)FRI Ex (4:45-5:45) ]

审审 Network: N101 [Pacific
Hwy-Wentworth Ave-FRI EX
(4:45-5:45)]
Staged crossing Stage 1 (Minor Road) at three-way intersection with 5-lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand <br> Total veh/h | Flows <br> HV <br> \% | Arriva <br> Total veh/h | $\begin{gathered} \text { Flows } \\ \mathrm{HV} \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service |  | ck of istance m | Prop. Queued | Effective Stop Rate |  | Averag e Speed km/h |
| South: Minor Road (Stage 1) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 122 | 0.9 | 122 | 0.9 | 0.168 | 10.9 | LOS A | 0.6 | 4.4 | 0.49 | 0.95 | 0.49 | 47.7 |
| 2 T1 | 26 | 0.0 | 26 | 0.0 | 0.128 | 24.8 | LOS B | 0.4 | 2.9 | 0.83 | 1.00 | 0.83 | 34.3 |
| Approach | 148 | 0.7 | 148 | 0.7 | 0.168 | 13.4 | LOS A | 0.6 | 4.4 | 0.55 | 0.96 | 0.55 | 45.4 |
| East: Major Road East |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 44 | 0.0 | 44 | 0.0 | 0.243 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.06 | 0.00 | 57.8 |
| $5 \quad \mathrm{~T} 1$ | 878 | 4.4 | 878 | 4.4 | 0.243 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.03 | 0.00 | 59.6 |
| 6 R2 | 1 | 0.0 | 1 | 0.0 | 0.001 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 50.6 |
| Approach | 923 | 4.2 | 923 | 4.2 | 0.243 | 0.3 | NA | 0.0 | 0.0 | 0.00 | 0.03 | 0.00 | 59.5 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 12 R 2 | 172 | 1.2 | 172 | 1.2 | 0.368 | 14.2 | LOSA | 1.5 | 10.3 | 0.76 | 0.95 | 0.95 | 44.4 |
| 12u U | 3 | 33.3 | 3 | 33.3 | 0.368 | 18.5 | LOS B | 1.5 | 10.3 | 0.76 | 0.95 | 0.95 | 34.7 |
| Approach | 175 | 1.8 | 175 | 1.8 | 0.368 | 14.2 | NA | 1.5 | 10.3 | 0.76 | 0.95 | 0.95 | 44.2 |
| All Vehicles | 1246 |  | 1246 | 3.5 | 0.368 | 3.8 | NA | 1.5 | 10.3 | 0.17 | 0.27 | 0.20 | 54.9 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

जT0F Site: 1 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road)FRI Ex (7:30-8:30) ]

审审 Network: N101 [Pacific
Hwy-Wentworth Ave-FRI EX (7:30-8:30)]

Staged crossing Stage 1 (Minor Road) at three-way intersection with 5-lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn | Demand Flows Arrival Flows |  |  |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Queue Vehicles Distance veh |  | Prop. Queued | Effective Stop Rate | Aver. Averag No. <br> Cycles Speed <br> km/h |  |
|  | Total veh/h |  |  | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ |  |  |  |  |  |  |  |  |  |
| South: Minor Road (Stage 1) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L 2 | 64 | 1.6 | 64 | 1.6 | 0.127 | 13.5 | LOSA | 0.4 | 3.1 | 0.61 | 1.00 | 0.61 | 45.8 |
| 2 T1 | 13 | 0.0 | 13 | 0.0 | 0.136 | 46.0 | LOS D | 0.4 | 2.8 | 0.92 | 1.00 | 0.92 | 24.7 |
| Approach | 77 | 1.4 | 77 | 1.4 | 0.136 | 18.8 | LOS B | 0.4 | 3.1 | 0.66 | 1.00 | 0.66 | 41.6 |
| East: Major Road East |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 33 | 3.2 | 33 | 3.2 | 0.355 | 5.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.03 | 0.00 | 57.9 |
| $5 \quad \mathrm{~T} 1$ | 1321 | 3.3 | 1321 | 3.3 | 0.355 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 59.7 |
| 6 R2 | 1 | 100.0 | 1 | $\begin{array}{r} 100 . \\ 0 \\ \hline \end{array}$ | 0.001 | 6.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 50.6 |
| Approach | 1355 | 3.4 | 1355 | 3.4 | 0.355 | 0.2 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 59.7 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 12 R 2 | 66 | 0.0 | 66 | 0.0 | 0.296 | 23.1 | LOS B | 0.9 | 6.4 | 0.88 | 0.98 | 0.99 | 38.8 |
| 12u U | 3 | 0.0 | 3 | 0.0 | 0.296 | 23.4 | LOS B | 0.9 | 6.4 | 0.88 | 0.98 | 0.99 | 32.4 |
| Approach | 69 | 0.0 | 69 | 0.0 | 0.296 | 23.1 | NA | 0.9 | 6.4 | 0.88 | 0.98 | 0.99 | 38.5 |
| All Vehicles | 1501 | 3.2 | 1501 | 3.2 | 0.355 | 2.2 | NA | 0.9 | 6.4 | 0.07 | 0.11 | 0.08 | 57.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: TTPP - THE TRANSPORT PLANNING PARTNERSHIP | Processed: Wednesday, 10 October 2018 3:57:48 PM
Project: X:117395 Doyalson Wyee RSLl07 Modelling Files\190614\17395_190614_Sc1 Existing.sip8

## MOVEMENT SUMMARY

Grof Site: 1 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road)SAT Ex (11:00-12:00) ]

审审 Network: N101 [Pacific Hwy-Wentworth Ave-SAT EX (11:00-12:00) ]

Staged crossing Stage 1 (Minor Road) at three-way intersection with 5-lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn | Demand Flows Arrival Flows |  |  |  | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \end{aligned}$ | Average Delay | Level of Service | 95\% Back of Queue Vehicles Distance |  | Prop. Queued | Effective Stop Rate | Aver. Averag No. e <br> Cycles Speed |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | Total veh/h |  |  | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | v/c | sec |  | Vehicles veh | m |  |  | Cycles | Speed km/h |
| South: Minor Road (Stage 1) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 111 | 1.0 | 111 | 1.0 | 0.159 | 11.2 | LOS A | 0.6 | 4.1 | 0.51 | 0.96 | 0.51 | 47.5 |
| $2 \quad \mathrm{~T} 1$ | 42 | 0.0 | 42 | 0.0 | 0.214 | 27.0 | LOS B | 0.7 | 5.1 | 0.85 | 1.01 | 0.90 | 33.0 |
| Approach | 153 | 0.7 | 153 | 0.7 | 0.214 | 15.6 | LOS B | 0.7 | 5.1 | 0.60 | 0.98 | 0.62 | 43.5 |
| East: Major Road East |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 38 | 0.0 | 38 | 0.0 | 0.257 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.05 | 0.00 | 57.9 |
| $5 \quad \mathrm{~T} 1$ | 942 | 3.0 | 942 | 3.0 | 0.257 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 59.7 |
| 6 R2 | 1 | 0.0 | 1 | 0.0 | 0.001 | 5.7 | LOSA | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 50.6 |
| Approach | 981 | 2.9 | 981 | 2.9 | 0.257 | 0.2 | NA | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 59.6 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 12 R 2 | 124 | 0.0 | 124 | 0.0 | 0.280 | 14.0 | LOS A | 1.0 | 6.9 | 0.75 | 0.92 | 0.86 | 44.6 |
| 12u U | 1 | 0.0 | 1 | 0.0 | 0.280 | 14.6 | LOS B | 1.0 | 6.9 | 0.75 | 0.92 | 0.86 | 38.8 |
| Approach | 125 | 0.0 | 125 | 0.0 | 0.280 | 14.0 | NA | 1.0 | 6.9 | 0.75 | 0.92 | 0.86 | 44.5 |
| All Vehicles | 1259 | 2.3 | 1259 | 2.3 | 0.280 | 3.5 | NA | 1.0 | 6.9 | 0.15 | 0.23 | 0.16 | 55.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

$\nabla$ site: 2 [Pacific Hwy-Wentworth Ave-Stage 2 (Median) NSWFRI Ex (4:45-5:45) ]

审审 Network: N101 [Pacific
Hwy-Wentworth Ave-FRI EX (4:45-5:45)]

Staged crossing Stage 2 (Median) at three-way intersection with 5 -lane major road.
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)


Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

$\nabla$ Site: 2 [Pacific Hwy-Wentworth Ave-Stage 2 (Median) NSWFRI Ex (7:30-8:30) ]

审审 Network: N101 [Pacific
Hwy-Wentworth Ave-FRI EX (7:30-8:30)]
Staged crossing Stage 2 (Median) at three-way intersection with 5 -lane major road.
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn Demand Flows Arrival Flows |  |  |  |  | Deg. <br> Satn <br> v/c | Average Delay sec | Level of Service | 95\% Back of Queue <br> Vehicles Distance veh |  | Prop. Queued | Effective Stop Rate | Aver. Averag No. e Cycles Speed km/h |  |
|  | Total veh/h |  |  | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ |  |  |  |  |  |  |  |  |  |
| South: Median Storage Area |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 R2 | 13 | 0.0 | 13 | 0.0 | 0.019 | 3.4 | LOS A | 0.1 | 0.3 | 0.54 | 0.46 | 0.54 | 48.3 |
| Approach | 13 | 0.0 | 13 | 0.0 | 0.019 | 3.4 | LOSA | 0.1 | 0.3 | 0.54 | 0.46 | 0.54 | 48.3 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 T1 | 904 | 8.8 | 904 | 8.8 | 0.245 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| Approach | 904 | 8.8 | 904 | 8.8 | 0.245 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| All Vehicles | 917 | 8.7 | 917 | 8.7 | 0.245 | 0.1 | NA | 0.1 | 0.3 | 0.01 | 0.01 | 0.01 | 59.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

$\nabla$ site: 2 [Pacific Hwy-Wentworth Ave-Stage 2 (Median) NSWSAT Ex (11:00-12:00) ]

审审 Network: N101 [Pacific Hwy-Wentworth Ave-SAT EX (11:00-12:00) ]
Staged crossing Stage 2 (Median) at three-way intersection with 5 -lane major road
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)


Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

## Site: 101 [Pacific Hwy-Rutley Rd Ex (4:45-5:45)]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 100 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Pacific Hwy - E |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 897 | 2.9 | 0.234 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| 6 | R2 | 218 | 1.9 | 0.382 | 30.2 | LOS C | 8.2 | 58.5 | 0.83 | 0.85 | 0.93 | 46.0 |
| Appr |  | 1115 | 2.7 | 0.382 | 5.9 | LOS A | 8.2 | 58.5 | 0.16 | 0.17 | 0.18 | 69.8 |
| North: Rutley Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 327 | 0.0 | 0.176 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 66.2 |
| 9 | R2 | 215 | 2.5 | 0.840 | 59.9 | LOS E | 11.5 | 82.5 | 1.00 | 0.92 | 1.28 | 33.4 |
| Approach |  | 542 | 1.0 | 0.840 | 28.3 | LOS B | 11.5 | 82.5 | 0.40 | 0.73 | 0.51 | 47.8 |
| West: Pacific Hwy - W |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 227 | 3.2 | 0.210 | 11.9 | LOS A | 3.3 | 23.7 | 0.41 | 0.71 | 0.41 | 59.8 |
| 11 | T1 | 1176 | 1.7 | 0.693 | 24.2 | LOS B | 23.0 | 163.6 | 0.87 | 0.77 | 0.87 | 52.3 |
| Appr |  | 1403 | 2.0 | 0.693 | 22.2 | LOS B | 23.0 | 163.6 | 0.79 | 0.76 | 0.79 | 53.4 |
| All V | icles | 3060 | 2.1 | 0.840 | 17.4 | LOS B | 23.0 | 163.6 | 0.49 | 0.54 | 0.52 | 57.1 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: X:\17395 Doyalson Wyee RSLl07 Modelling Files\190614\17395_190614_Sc1 Existing.sip8

## MOVEMENT SUMMARY

## Site: 101 [Pacific Hwy-Rutley Rd Ex (7:30-8:30)]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 102 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman <br> Total veh/h | $\begin{gathered} \text { Flows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. <br> Satn <br> V/C | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Pacific Hwy - E 0.0 0.0 |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 1242 | 4.1 | 0.327 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| 6 | R2 | 220 | 5.7 | 0.378 | 22.8 | LOS B | 7.1 | 52.2 | 0.73 | 0.80 | 0.73 | 50.0 |
| Appr |  | 1462 | 4.3 | 0.378 | 3.5 | LOS A | 7.1 | 52.2 | 0.11 | 0.12 | 0.11 | 73.2 |
| North: Rutley Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 204 | 6.7 | 0.115 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 64.1 |
| 9 | R2 | 228 | 7.4 | 0.734 | 53.0 | LOS D | 11.4 | 85.2 | 1.00 | 0.86 | 1.10 | 35.2 |
| Approach |  | 433 | 7.1 | 0.734 | 31.6 | LOS C | 11.4 | 85.2 | 0.53 | 0.74 | 0.58 | 44.8 |
| West: Pacific Hwy - W |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 125 | 10.1 | 0.120 | 11.0 | LOS A | 1.6 | 12.2 | 0.34 | 0.68 | 0.34 | 58.9 |
| 11 | T1 | 758 | 7.4 | 0.452 | 20.7 | LOS B | 12.9 | 96.2 | 0.74 | 0.64 | 0.74 | 55.1 |
| Appr |  | 883 | 7.7 | 0.452 | 19.3 | LOS B | 12.9 | 96.2 | 0.68 | 0.65 | 0.68 | 55.6 |
| All V | icles | 2778 | 5.8 | 0.734 | 12.9 | LOS A | 12.9 | 96.2 | 0.36 | 0.38 | 0.36 | 61.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: X:\17395 Doyalson Wyee RSLl07 Modelling Files\190614\17395_190614_Sc1 Existing.sip8

## MOVEMENT SUMMARY

## Site: 101 [Pacific Hwy-Rutley Rd Ex SAT (11:15-12:15)]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 102 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. <br> Satn <br> v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Pacific Hwy - E |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 919 | 1.6 | 0.238 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| 6 | R2 | 175 | 3.0 | 0.276 | 21.4 | LOS B | 5.3 | 37.9 | 0.67 | 0.78 | 0.67 | 51.5 |
| Appr |  | 1094 | 1.8 | 0.276 | 3.4 | LOS A | 5.3 | 37.9 | 0.11 | 0.12 | 0.11 | 73.4 |
| North: Rutley Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 188 | 1.7 | 0.103 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 65.7 |
| 9 | R2 | 164 | 1.9 | 0.653 | 54.2 | LOS D | 8.1 | 57.8 | 1.00 | 0.82 | 1.04 | 35.3 |
| Approach |  | 353 | 1.8 | 0.653 | 29.3 | LOS C | 8.1 | 57.8 | 0.46 | 0.71 | 0.49 | 47.0 |
| West: Pacific Hwy - W |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 174 | 1.2 | 0.149 | 9.9 | LOS A | 1.9 | 13.4 | 0.30 | 0.68 | 0.30 | 62.3 |
| 11 | T1 | 918 | 2.3 | 0.541 | 22.4 | LOS B | 16.7 | 119.1 | 0.79 | 0.69 | 0.79 | 53.6 |
| Appr |  | 1092 | 2.1 | 0.541 | 20.4 | LOS B | 16.7 | 119.1 | 0.71 | 0.69 | 0.71 | 54.9 |
| All V | icles | 2538 | 1.9 | 0.653 | 14.3 | LOS A | 16.7 | 119.1 | 0.42 | 0.45 | 0.42 | 60.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: X:\17395 Doyalson Wyee RSLl07 Modelling Files\190614\17395_190614_Sc1 Existing.sip8

## MOVEMENT SUMMARY

## Site: 101 [Pacific HIghway-Wyee Road-Scenic Drive -AM 2018 Stage 1]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 105 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { =lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 463 | 1.6 | 0.252 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.9 |
| 22 | T1 | 155 | 8.2 | 0.893 | 60.0 | LOS E | 9.0 | 67.1 | 0.97 | 1.02 | 1.47 | 30.4 |
| 23 | R2 | 226 | 3.3 | 0.960 | 81.0 | LOS F | 15.2 | 109.3 | 1.00 | 1.11 | 1.65 | 21.6 |
| Appro |  | 844 | 3.2 | 0.960 | 35.8 | LOS C | 15.2 | 109.3 | 0.45 | 0.77 | 0.71 | 37.0 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 109 | 4.8 | 0.107 | 12.7 | LOS A | 1.5 | 10.7 | 0.48 | 0.71 | 0.48 | 48.5 |
| 25 | T1 | 1505 | 2.7 | 0.976 | 69.1 | LOS E | 55.5 | 397.4 | 1.00 | 1.21 | 1.45 | 26.4 |
| 26 | R2 | 40 | 0.0 | 0.565 | 66.2 | LOS E | 2.2 | 15.6 | 1.00 | 0.75 | 1.09 | 24.8 |
| Appro |  | 1655 | 2.7 | 0.976 | 65.3 | LOS E | 55.5 | 397.4 | 0.97 | 1.17 | 1.38 | 27.2 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 53 | 4.0 | 0.720 | 54.2 | LOS D | 8.4 | 62.2 | 1.00 | 0.91 | 1.39 | 28.8 |
| 28 | T1 | 122 | 7.8 | 0.720 | 48.5 | LOS D | 8.4 | 62.2 | 1.00 | 0.91 | 1.39 | 33.0 |
| 29 | R2 | 179 | 4.1 | 0.843 | 62.1 | LOS E | 10.1 | 73.1 | 1.00 | 0.95 | 1.31 | 30.7 |
| Appr |  | 354 | 5.4 | 0.843 | 56.3 | LOS D | 10.1 | 73.1 | 1.00 | 0.93 | 1.35 | 31.2 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 99 | 14.9 | 0.059 | 7.8 | LOS A | 0.0 | 0.0 | 0.00 | 0.59 | 0.00 | 61.8 |
| 31 | T1 | 934 | 7.6 | 0.527 | 20.6 | LOS B | 16.6 | 123.8 | 0.75 | 0.66 | 0.75 | 49.9 |
| 32 | R2 | 169 | 8.7 | 0.848 | 65.2 | LOS E | 9.7 | 72.9 | 1.00 | 0.93 | 1.33 | 30.6 |
| Approach |  | 1202 | 8.3 | 0.848 | 25.8 | LOS B | 16.6 | 123.8 | 0.72 | 0.70 | 0.77 | 45.7 |
| All Vehicles |  | 4055 | 4.7 | 0.976 | 46.7 | LOS D | 55.5 | 397.4 | 0.79 | 0.93 | 1.06 | 33.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^6]
## MOVEMENT SUMMARY

## Site: 101 [Pacific HIghway-Wyee Road-Scenic Drive -PM 2018 Stage 1]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 184 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 232 | 3.2 | 0.128 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.8 |
| 22 | T1 | 155 | 2.0 | 0.880 | 90.7 | LOS F | 14.3 | 101.7 | 0.95 | 0.94 | 1.25 | 24.3 |
| 23 | R2 | 224 | 1.9 | 0.998 | 137.0 | LOS F | 25.7 | 182.5 | 1.00 | 1.06 | 1.52 | 15.0 |
| Appro |  | 611 | 2.4 | 0.998 | 75.4 | LOS F | 25.7 | 182.5 | 0.61 | 0.83 | 0.87 | 25.5 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 199 | 2.1 | 0.263 | 27.4 | LOS B | 7.4 | 53.0 | 0.70 | 0.77 | 0.70 | 38.5 |
| 25 | T1 | 945 | 3.8 | 1.123 | 208.2 | LOS F | 75.4 | 545.0 | 1.00 | 1.41 | 1.79 | 10.9 |
| 26 | R2 | 57 | 0.0 | 0.352 | 93.8 | LOS F | 4.9 | 34.3 | 0.98 | 0.76 | 0.98 | 19.8 |
| Appr |  | 1201 | 3.3 | 1.123 | 172.8 | LOS F | 75.4 | 545.0 | 0.95 | 1.27 | 1.57 | 12.7 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 44 | 2.4 | 0.755 | 107.5 | LOS F | 21.3 | 150.6 | 1.00 | 0.94 | 1.62 | 18.6 |
| 28 | T1 | 195 | 1.1 | 0.755 | 101.9 | LOS F | 21.3 | 150.6 | 1.00 | 0.94 | 1.62 | 22.5 |
| 29 | R2 | 166 | 0.6 | 0.857 | 93.5 | LOS F | 15.2 | 106.7 | 0.97 | 0.91 | 1.20 | 24.5 |
| Appr |  | 405 | 1.0 | 0.857 | 99.1 | LOS F | 21.3 | 150.6 | 0.99 | 0.93 | 1.45 | 22.8 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 121 | 2.6 | 0.066 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 65.4 |
| 31 | T1 | 1457 | 1.8 | 1.035 | 130.0 | LOS F | 108.6 | 771.7 | 1.00 | 1.26 | 1.44 | 16.4 |
| 32 | R2 | 467 | 1.1 | 1.016 | 105.9 | LOS F | 40.4 | 285.4 | 1.00 | 1.03 | 1.44 | 18.6 |
| Approach |  | 2045 | 1.7 | 1.035 | 117.3 | LOS F | 108.6 | 771.7 | 0.94 | 1.17 | 1.35 | 18.0 |
| All Ve | icles | 4262 | 2.2 | 1.123 | 125.2 | LOS F | 108.6 | 771.7 | 0.90 | 1.13 | 1.36 | 17.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^7]
## MOVEMENT SUMMARY

## Site: 101 [Pacific HIghway-Wyee Road-Scenic Drive -Sat 2018 Stage 1]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 157 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 309 | 1.0 | 0.168 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.9 |
| 22 | T1 | 137 | 3.1 | 0.827 | 72.2 | LOS F | 10.3 | 74.3 | 0.94 | 0.88 | 1.19 | 27.6 |
| 23 | R2 | 226 | 1.9 | 0.923 | 93.6 | LOS F | 19.7 | 139.9 | 1.00 | 0.99 | 1.36 | 19.7 |
| Appro |  | 673 | 1.7 | 0.923 | 48.8 | LOS D | 19.7 | 139.9 | 0.53 | 0.76 | 0.70 | 32.2 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 182 | 1.2 | 0.210 | 20.8 | LOS B | 5.1 | 35.8 | 0.62 | 0.75 | 0.62 | 42.5 |
| 25 | T1 | 1017 | 2.4 | 0.966 | 90.4 | LOS F | 50.4 | 360.1 | 1.00 | 1.11 | 1.33 | 21.9 |
| 26 | R2 | 48 | 4.3 | 0.603 | 92.0 | LOS F | 3.9 | 28.3 | 1.00 | 0.76 | 1.06 | 20.1 |
| Appr |  | 1247 | 2.3 | 0.966 | 80.3 | LOS F | 50.4 | 360.1 | 0.94 | 1.05 | 1.22 | 23.5 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 67 | 3.1 | 0.829 | 83.3 | LOS F | 15.0 | 107.7 | 1.00 | 0.98 | 1.48 | 22.0 |
| 28 | T1 | 129 | 3.3 | 0.829 | 77.7 | LOS F | 15.0 | 107.7 | 1.00 | 0.98 | 1.48 | 26.2 |
| 29 | R2 | 139 | 2.3 | 0.771 | 81.3 | LOS F | 10.7 | 76.6 | 1.00 | 0.87 | 1.14 | 26.5 |
| Appr |  | 336 | 2.8 | 0.829 | 80.3 | LOS F | 15.0 | 107.7 | 1.00 | 0.93 | 1.34 | 25.6 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 115 | 5.5 | 0.064 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 64.5 |
| 31 | T1 | 1043 | 1.6 | 0.593 | 27.5 | LOS B | 30.7 | 218.0 | 0.73 | 0.65 | 0.73 | 44.3 |
| 32 | R2 | 304 | 1.4 | 0.764 | 41.1 | LOS C | 12.1 | 85.9 | 1.00 | 0.86 | 1.04 | 38.4 |
| Approach |  | 1462 | 1.9 | 0.764 | 28.7 | LOS C | 30.7 | 218.0 | 0.73 | 0.69 | 0.74 | 43.9 |
| All Vehicles |  | 3718 | 2.1 | 0.966 | 54.3 | LOS D | 50.4 | 360.1 | 0.79 | 0.84 | 0.95 | 31.1 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^8]
## MOVEMENT SUMMARY

## sivi Site: 1 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road)AM 2018 Stage 1]

㿾审 Network: N101 [Pacific Hwy-Wentworth Ave-FRI AM 2018 Stage 1]
Staged crossing Stage 1 (Minor Road) at three-way intersection with 5-lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand <br> Total veh/h | Flows HV \% | Arriva Total veh/h | $\begin{gathered} =l o w s \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Que <br> $\begin{array}{c}\text { Vehicles } \\ \text { veh }\end{array}$ | of ance $m$ | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Averag Speed km/h |
| South: Minor Road (Stage 1) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 55 | 1.9 | 55 | 1.9 | 0.128 | 15.4 | LOS B | 0.5 | 3.3 | 0.67 | 1.00 | 0.67 | 44.4 |
| $2 \quad \mathrm{~T} 1$ | 13 | 0.0 | 13 | 0.0 | 0.155 | 52.4 | LOS D | 0.5 | 3.2 | 0.93 | 1.00 | 0.94 | 22.7 |
| Approach | 67 | 1.6 | 67 | 1.6 | 0.155 | 22.3 | LOS B | 0.5 | 3.3 | 0.72 | 1.00 | 0.72 | 39.3 |
| East: Major Road East |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 9 | 11.1 | 9 | 11.1 | 0.422 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 57.4 |
| $5 \quad \mathrm{~T} 1$ | 1607 | 2.8 | 1607 | 2.8 | 0.422 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.8 |
| 6 R2 | 1 | 100.0 | 1 | $\begin{array}{r} 100 . \\ 0 \end{array}$ | 0.001 | 6.4 | LOSA | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 48.2 |
| Approach | 1618 | 2.9 | 1618 | 2.9 | 0.422 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.8 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 12 R 2 | 13 | 0.0 | 13 | 0.0 | 0.082 | 24.3 | LOS B | 0.2 | 1.6 | 0.88 | 0.95 | 0.88 | 38.0 |
| 12u U | 3 | 0.0 | 3 | 0.0 | 0.082 | 25.2 | LOS B | 0.2 | 1.6 | 0.88 | 0.95 | 0.88 | 31.6 |
| Approach | 16 | 0.0 | 16 | 0.0 | 0.082 | 24.4 | NA | 0.2 | 1.6 | 0.88 | 0.95 | 0.88 | 37.0 |
| All Vehicles | 1701 |  | 1701 | 2.8 | 0.422 | 1.2 | NA | 0.5 | 3.3 | 0.04 | 0.05 | 0.04 | 58.1 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2019 Akcelik and Associates Pty Ltd | sidrasolutions.com
Organisation: TTPP - THE TRANSPORT PLANNING PARTNERSHIP | Processed: Friday, 7 December 2018 3:25:35 PM
Project: X:\17395 Doyalson Wyee RSLl07 Modelling Files\190614\17395_190617_Sc2 2018 +Stage 1.sip8

## MOVEMENT SUMMARY

## $\nabla$ site: 2 [Pacific Hwy-Wentworth Ave-Stage 2 (Median) NSWAM 2018 Stage 1]

审官 Network: N101 [Pacific Hwy-Wentworth Ave-FRI AM 2018 Stage 1]

Staged crossing Stage 2 (Median) at three-way intersection with 5 -lane major road.
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand Flows Arrival Flows |  |  |  | Deg. Satn <br> v/c | Average Delay sec | Level of Service | 95\% Back of Queue Vehicles Distance veh |  | Prop. Queued | Effective Stop Rate | Aver. Averag No. e Cycles Speed km/h |  |
| South: Median Storage Area |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 R2 | 13 | 0.0 | 13 | 0.0 | 0.026 | 5.7 | LOS A | 0.1 | 0.4 | 0.67 | 0.65 | 0.67 | 42.5 |
| Approach | 13 | 0.0 | 13 | 0.0 | 0.026 | 5.7 | LOS A | 0.1 | 0.4 | 0.67 | 0.65 | 0.67 | 42.5 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 T1 | 1203 | 6.6 | 1203 | 6.6 | 0.322 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| Approach | 1203 | 6.6 | 1203 | 6.6 | 0.322 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| All Vehicles | 1216 |  | 1216 | 6.6 | 0.322 | 0.1 | NA | 0.1 | 0.4 | 0.01 | 0.01 | 0.01 | 59.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

## STlif Site: 1 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road) PM 2018 Stage 1]

㿾审 Network: N101 [Pacific Hwy-Wentworth Ave-FRI PM 2018 Stage 1]

Staged crossing Stage 1 (Minor Road) at three-way intersection with 5-lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand <br> Total veh/h | Flows <br> HV <br> \% | Arrival <br> Total veh/h | $\begin{gathered} \text { Flows } \\ \mathrm{HV} \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service |  | Back of eue Distance | Prop. Queued | Effective Stop Rate |  | Averag e Speed km/h |
| South: Minor Road (Stage 1) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 39 | 2.7 | 39 | 2.7 | 0.064 | 12.0 | LOS A | 0.2 | 1.7 | 0.54 | 0.93 | 0.54 | 46.7 |
| $2 \quad \mathrm{~T} 1$ | 6 | 0.0 | 6 | 0.0 | 0.034 | 26.3 | LOS B | 0.1 | 0.8 | 0.83 | 1.00 | 0.83 | 33.4 |
| Approach | 45 | 2.3 | 45 | 2.3 | 0.064 | 14.0 | LOS A | 0.2 | 1.7 | 0.58 | 0.94 | 0.58 | 44.9 |
| East: Major Road East |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 22 | 0.0 | 22 | 0.0 | 0.309 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 57.9 |
| $5 \quad \mathrm{~T} 1$ | 1155 | 3.4 | 1155 | 3.4 | 0.309 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 59.8 |
| 6 R2 | 1 | 0.0 | 1 | 0.0 | 0.001 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 48.2 |
| Approach | 1178 | 3.3 | 1178 | 3.3 | 0.309 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 59.7 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 12 R 2 | 69 | 3.0 | 69 | 3.0 | 0.194 | 15.0 | LOS B | 0.6 | 4.5 | 0.77 | 0.91 | 0.79 | 43.6 |
| 12u U | 3 | 33.3 | 3 | 33.3 | 0.194 | 20.4 | LOS B | 0.6 | 4.5 | 0.77 | 0.91 | 0.79 | 34.1 |
| Approach | 73 | 4.3 | 73 | 4.3 | 0.194 | 15.2 | NA | 0.6 | 4.5 | 0.77 | 0.91 | 0.79 | 43.3 |
| All Vehicles | 1296 | 3.3 | 1296 | 3.3 | 0.309 | 1.5 | NA | 0.6 | 4.5 | 0.06 | 0.09 | 0.06 | 57.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

$\nabla$ Site: 2 [Pacific Hwy-Wentworth Ave-Stage 2 (Median) NSW -
PM 2018 Stage 1]
审禹 Network: N101 [Pacific Hwy-Wentworth Ave-FRI PM 2018 Stage 1]
Staged crossing Stage 2 (Median) at three-way intersection with 5 -lane major road.
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand <br> Total veh/h | lows | Arriva <br> Total veh/h | lows <br> HV <br> \% | Deg. Satn v/c | Average Delay | Level of Service |  | of <br> tance <br> m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Averag <br> peed <br> km/h |
| South: Median Storage Area |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 R2 | 6 | 0.0 | 6 | 0.0 | 0.024 | 11.6 | LOS A | 0.1 | 0.4 | 0.82 | 0.82 | 0.82 | 36.2 |
| Approach | 6 | 0.0 | 6 | 0.0 | 0.024 | 11.6 | LOS A | 0.1 | 0.4 | 0.82 | 0.82 | 0.82 | 36.2 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 T1 | 1660 | 2.3 | 1660 | 2.3 | 0.432 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| Approach | 1660 | 2.3 | 1660 | 2.3 | 0.432 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| All Vehicles | 1666 | 2.3 | 1666 | 2.3 | 0.432 | 0.1 | NA | 0.1 | 0.4 | 0.00 | 0.00 | 0.00 | 59.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

siof Site: 1 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road) SAT 2018 Stage 1]

审审 Network: N101 [Pacific Hwy-Wentworth Ave-SAT SAT 2018 Stage 1]

Staged crossing Stage 1 (Minor Road) at three-way intersection with 5-lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn | Demand Flows Arrival Flows |  |  |  | Deg. <br> Satn | Average Delay | Level of Service | 95\% Back of Queue |  | Prop. Queued | Effective Stop Rate | Aver. Averag No. Cycles Speed |  |
|  | Total |  |  | HV |  |  |  | Vehicles D | Distance |  |  |  |  |
|  | veh/h |  | veh/h | \% | v/c | sec |  | veh | m |  |  |  | km/h |
| South: Minor Road (Stage 1) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 62 | 1.7 | 62 | 1.7 | 0.103 | 12.2 | LOSA | 0.4 | 2.7 | 0.55 | 0.96 | 0.55 | 46.7 |
| 2 T1 | 19 | 0.0 | 19 | 0.0 | 0.103 | 27.0 | LOS B | 0.3 | 2.4 | 0.84 | 1.00 | 0.84 | 33.0 |
| Approach | 81 | 1.3 | 81 | 1.3 | 0.103 | 15.7 | LOS B | 0.4 | 2.7 | 0.62 | 0.97 | 0.62 | 43.6 |
| East: Major Road East |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 19 | 0.0 | 19 | 0.0 | 0.314 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 57.9 |
| $5 \quad \mathrm{~T} 1$ | 1186 | 2.4 | 1186 | 2.4 | 0.314 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 59.8 |
| 6 R2 | 1 | 0.0 | 1 | 0.0 | 0.001 | 5.7 | LOSA | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 48.2 |
| Approach | 1206 | 2.4 | 1206 | 2.4 | 0.314 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 59.7 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 12 R 2 | 39 | 0.0 | 39 | 0.0 | 0.105 | 14.5 | LOS A | 0.3 | 2.3 | 0.76 | 0.90 | 0.76 | 44.2 |
| 12 u U | 1 | 0.0 | 1 | 0.0 | 0.105 | 15.4 | LOS B | 0.3 | 2.3 | 0.76 | 0.90 | 0.76 | 38.4 |
| Approach | 40 | 0.0 | 40 | 0.0 | 0.105 | 14.5 | NA | 0.3 | 2.3 | 0.76 | 0.90 | 0.76 | 44.1 |
| All Vehicles | 1327 | 2.2 | 1327 | 2.2 | 0.314 | 1.5 | NA | 0.4 | 2.7 | 0.06 | 0.10 | 0.06 | 57.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

$\nabla$ site: 2 [Pacific Hwy-Wentworth Ave-Stage 2 (Median) NSW SAT 2018 Stage 1 ]

审审 Network: N101 [Pacific
Hwy-Wentworth Ave-SAT SAT 2018 Stage 1]

Staged crossing Stage 2 (Median) at three-way intersection with 5 -lane major road.
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | Demand Flows Arrival Flows |  |  |  | Deg. Satn v/c | Average Delay | Level of Service | 95\% Back of Queue Vehicles Distance veh m |  | Prop. Queued | Effective Stop Rate | Aver. Averag No. <br> Cycles Speed km/h |  |
|  | Tota | HV | Total | HV |  |  |  |  |  |  |  |  |  |
|  | veh/h |  | veh/h | \% |  |  |  |  |  |  |  |  |  |
| South: Median Storage Area |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 R2 | 19 | 0.0 | 19 | 0.0 | 0.042 | 6.4 | LOS A | 0.1 | 0.7 | 0.70 | 0.70 | 0.70 | 41.6 |
| Approach | 19 | 0.0 | 19 | 0.0 | 0.042 | 6.4 | LOS A | 0.1 | 0.7 | 0.70 | 0.70 | 0.70 | 41.6 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 T1 | 1294 | 1.8 | 1294 | 1.8 | 0.336 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| Approach | 1294 | 1.8 | 1294 | 1.8 | 0.336 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| All Vehicles | 1313 | 1.8 | 1313 | 1.8 | 0.336 | 0.1 | NA | 0.1 | 0.7 | 0.01 | 0.01 | 0.01 | 59.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

## Site: 101 [Pacific Hwy-Rutley Rd -AM 2018 Stage 1]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 102 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | Turn | Deman <br> Total veh/h | $\begin{gathered} \text { Flows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. <br> Satn <br> V/C | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Pacific Hwy - E 0.0 0.0 0 en |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 1323 | 3.8 | 0.348 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.8 |
| 6 | R2 | 220 | 5.7 | 0.398 | 24.9 | LOS B | 7.7 | 56.9 | 0.78 | 0.81 | 0.78 | 48.6 |
| Appr | ach | 1543 | 4.1 | 0.398 | 3.6 | LOS A | 7.7 | 56.9 | 0.11 | 0.12 | 0.11 | 73.1 |
| North: Rutley Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 204 | 6.7 | 0.115 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 64.1 |
| 9 | R2 | 228 | 7.4 | 0.734 | 53.0 | LOS D | 11.4 | 85.2 | 1.00 | 0.86 | 1.10 | 35.2 |
| Approach |  | 433 | 7.1 | 0.734 | 31.6 | LOS C | 11.4 | 85.2 | 0.53 | 0.74 | 0.58 | 44.8 |
| West: Pacific Hwy - W |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 125 | 10.1 | 0.123 | 11.9 | LOS A | 1.8 | 13.7 | 0.37 | 0.68 | 0.37 | 58.1 |
| 11 | T1 | 851 | 6.6 | 0.504 | 21.3 | LOS B | 15.0 | 110.6 | 0.76 | 0.67 | 0.76 | 54.5 |
| Appr | ach | 976 | 7.0 | 0.504 | 20.1 | LOS B | 15.0 | 110.6 | 0.71 | 0.67 | 0.71 | 55.0 |
| All V | hicles | 2952 | 5.5 | 0.734 | 13.2 | LOS A | 15.0 | 110.6 | 0.37 | 0.39 | 0.38 | 60.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2019 Akcelik and Associates Pty Ltd | sidrasolutions.com
Organisation: TTPP - THE TRANSPORT PLANNING PARTNERSHIP | Processed: Friday, 7 December 2018 3:26:27 PM
Project: X:I17395 Doyalson Wyee RSLl07 Modelling Filesl190614\17395_190617_Sc2 2018 +Stage 1.sip8

## MOVEMENT SUMMARY

## Site: 101 [Pacific Hwy-Rutley Rd -PM 2018 Stage 1]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 100 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/C | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Pacific Hwy - E |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 972 | 2.7 | 0.254 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| 6 | R2 | 218 | 1.9 | 0.390 | 32.1 | LOS C | 8.3 | 58.9 | 0.84 | 0.86 | 0.99 | 45.0 |
| Appr |  | 1189 | 2.6 | 0.390 | 5.9 | LOS A | 8.3 | 58.9 | 0.15 | 0.16 | 0.18 | 69.9 |
| North: Rutley Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 327 | 0.0 | 0.176 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 66.2 |
| 9 | R2 | 215 | 2.5 | 0.840 | 59.9 | LOS E | 11.5 | 82.5 | 1.00 | 0.92 | 1.28 | 33.4 |
| Approach |  | 542 | 1.0 | 0.840 | 28.3 | LOS B | 11.5 | 82.5 | 0.40 | 0.73 | 0.51 | 47.8 |
| West: Pacific Hwy - W |  |  |  |  |  |  |  |  |  |  |  |  |
|  | L2 | 227 | 3.2 | 0.205 | 11.3 | LOS A | 3.0 | 21.9 | 0.39 | 0.70 | 0.39 | 60.4 |
|  | T1 | 1240 | 1.6 | 0.730 | 24.8 | LOS B | 24.9 | 176.7 | 0.89 | 0.80 | 0.89 | 51.8 |
| Approach |  | 1467 | 1.9 | 0.730 | 22.7 | LOS B | 24.9 | 176.7 | 0.81 | 0.78 | 0.81 | 53.0 |
| All Vehicles |  | 3199 | 2.0 | 0.840 | 17.4 | LOS B | 24.9 | 176.7 | 0.50 | 0.54 | 0.53 | 57.1 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

## Site: 101 [Pacific Hwy-Rutley Rd -SAT 2018 Stage 1]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 102 seconds (Site User-Given Phase Times)


Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

## Site: 101 [Pacific HIghway-Wyee Road-Scenic Drive -FRI AM 2028 Base]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 104 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Demand <br> Total veh/h | $\begin{gathered} \text { Flows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 482 | 1.6 | 0.263 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.9 |
| 22 | T1 | 161 | 8.2 | 0.920 | 64.7 | LOS E | 9.7 | 72.6 | 0.98 | 1.06 | 1.57 | 29.3 |
| 23 | R2 | 188 | 4.1 | 0.716 | 53.4 | LOS D | 9.6 | 69.5 | 1.00 | 0.86 | 1.09 | 27.7 |
| Appro |  | 832 | 3.4 | 0.920 | 27.9 | LOS B | 9.7 | 72.6 | 0.41 | 0.71 | 0.55 | 40.5 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 104 | 5.7 | 0.115 | 14.4 | LOS A | 1.6 | 11.4 | 0.55 | 0.72 | 0.55 | 47.1 |
| 25 | T1 | 1429 | 3.2 | 1.086 | 142.5 | LOS F | 73.7 | 529.9 | 1.00 | 1.58 | 2.07 | 15.2 |
| 26 | R2 | 38 | 0.0 | 0.424 | 63.2 | LOS E | 2.0 | 14.2 | 1.00 | 0.73 | 1.00 | 25.6 |
| Appr |  | 1572 | 3.3 | 1.086 | 132.1 | LOS F | 73.7 | 529.9 | 0.97 | 1.50 | 1.94 | 16.1 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 48 | 5.0 | 0.635 | 51.0 | LOS D | 8.6 | 63.7 | 0.99 | 0.86 | 1.30 | 29.8 |
| 28 | T1 | 139 | 7.8 | 0.635 | 45.3 | LOS D | 8.6 | 63.7 | 0.99 | 0.86 | 1.30 | 34.1 |
| 29 | R2 | 204 | 4.1 | 0.840 | 59.2 | LOS E | 11.2 | 81.3 | 1.00 | 0.95 | 1.29 | 31.4 |
| Appr |  | 392 | 5.5 | 0.840 | 53.2 | LOS D | 11.2 | 81.3 | 0.99 | 0.91 | 1.29 | 32.2 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 125 | 14.9 | 0.075 | 7.8 | LOS A | 0.0 | 0.0 | 0.00 | 0.59 | 0.00 | 61.8 |
| 31 | T1 | 946 | 9.5 | 0.595 | 24.2 | LOS B | 18.3 | 138.4 | 0.82 | 0.72 | 0.82 | 46.8 |
| 32 | R2 | 215 | 8.7 | 0.851 | 63.0 | LOS E | 12.1 | 91.2 | 1.00 | 0.93 | 1.30 | 31.2 |
| Approach |  | 1286 | 9.9 | 0.851 | 29.1 | LOS C | 18.3 | 138.4 | 0.77 | 0.75 | 0.82 | 43.6 |
| All Ve | icles | 4081 | 5.6 | 1.086 | 70.8 | LOS F | 73.7 | 529.9 | 0.80 | 1.05 | 1.24 | 26.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^9]
## MOVEMENT SUMMARY

## Site: 102 [Pacific HIghway-Wyee Road-Scenic Drive -FRI PM 2028 Base]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 184 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 264 | 3.2 | 0.145 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.8 |
| 22 | T1 | 177 | 2.1 | 1.079 | 187.3 | LOS F | 24.8 | 176.6 | 1.00 | 1.27 | 1.80 | 14.5 |
| 23 | R2 | 206 | 2.3 | 1.051 | 171.7 | LOS F | 27.2 | 194.2 | 1.00 | 1.13 | 1.68 | 12.4 |
| Appro |  | 647 | 2.6 | 1.079 | 108.2 | LOS F | 27.2 | 194.2 | 0.59 | 0.92 | 1.03 | 20.2 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 219 | 2.3 | 0.283 | 26.7 | LOS B | 8.0 | 56.8 | 0.69 | 0.78 | 0.69 | 38.9 |
| 25 | T1 | 908 | 4.7 | 1.002 | 125.4 | LOS F | 57.7 | 419.8 | 1.00 | 1.16 | 1.41 | 17.1 |
| 26 | R2 | 62 | 0.0 | 0.410 | 95.5 | LOS F | 5.4 | 38.0 | 0.99 | 0.76 | 0.99 | 19.6 |
| Appr |  | 1189 | 4.0 | 1.002 | 105.6 | LOS F | 57.7 | 419.8 | 0.94 | 1.07 | 1.25 | 19.2 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 38 | 3.0 | 0.941 | 144.8 | LOS F | 29.5 | 209.1 | 1.00 | 1.13 | 2.01 | 14.9 |
| 28 | T1 | 211 | 1.1 | 0.941 | 139.2 | LOS F | 29.5 | 209.1 | 1.00 | 1.13 | 2.01 | 18.3 |
| 29 | R2 | 180 | 0.6 | 1.102 | 207.8 | LOS F | 26.2 | 184.1 | 1.00 | 1.20 | 1.86 | 13.5 |
| Appr |  | 428 | 1.1 | 1.102 | 168.5 | LOS F | 29.5 | 209.1 | 1.00 | 1.16 | 1.94 | 15.7 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 145 | 2.6 | 0.080 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 65.4 |
| 31 | T1 | 1541 | 2.0 | 1.023 | 118.2 | LOS F | 113.9 | 811.4 | 1.00 | 1.22 | 1.39 | 17.8 |
| 32 | R2 | 561 | 1.1 | 1.153 | 207.4 | LOS F | 68.0 | 480.8 | 1.00 | 1.19 | 1.87 | 12.1 |
| Approach |  | 2247 | 1.9 | 1.153 | 133.3 | LOS F | 113.9 | 811.4 | 0.94 | 1.18 | 1.42 | 16.4 |
| All Ve | icles | 4513 | 2.4 | 1.153 | 125.8 | LOS F | 113.9 | 811.4 | 0.89 | 1.11 | 1.37 | 17.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^10]
## MOVEMENT SUMMARY

## Site: 103 [Pacific HIghway-Wyee Road-Scenic Drive -SAT 2028 Base]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 157 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 353 | 1.0 | 0.191 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.9 |
| 22 | T1 | 156 | 3.1 | 0.942 | 95.3 | LOS F | 13.8 | 98.9 | 0.95 | 1.03 | 1.45 | 23.6 |
| 23 | R2 | 217 | 2.2 | 0.913 | 91.2 | LOS F | 18.5 | 132.1 | 1.00 | 0.98 | 1.34 | 20.0 |
| Appro |  | 725 | 1.8 | 0.942 | 50.5 | LOS D | 18.5 | 132.1 | 0.50 | 0.77 | 0.71 | 31.8 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 182 | 1.4 | 0.219 | 21.6 | LOS B | 5.1 | 35.9 | 0.64 | 0.76 | 0.64 | 42.0 |
| 25 | T1 | 1009 | 2.8 | 1.024 | 125.7 | LOS F | 58.8 | 421.3 | 1.00 | 1.24 | 1.54 | 16.9 |
| 26 | R2 | 48 | 5.1 | 0.606 | 92.1 | LOS F | 3.9 | 28.6 | 1.00 | 0.76 | 1.07 | 20.1 |
| Appr |  | 1240 | 2.7 | 1.024 | 109.1 | LOS F | 58.8 | 421.3 | 0.95 | 1.15 | 1.39 | 18.6 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 61 | 3.7 | 0.851 | 87.8 | LOS F | 15.8 | 113.5 | 1.00 | 1.00 | 1.55 | 21.3 |
| 28 | T1 | 140 | 3.3 | 0.851 | 82.2 | LOS F | 15.8 | 113.5 | 1.00 | 1.00 | 1.55 | 25.4 |
| 29 | R2 | 151 | 2.3 | 0.895 | 92.4 | LOS F | 12.7 | 90.5 | 1.00 | 0.97 | 1.36 | 24.6 |
| Appr |  | 352 | 2.9 | 0.895 | 87.5 | LOS F | 15.8 | 113.5 | 1.00 | 0.99 | 1.47 | 24.4 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 138 | 5.5 | 0.077 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 64.5 |
| 31 | T1 | 1060 | 1.9 | 0.635 | 27.9 | LOS B | 33.9 | 241.1 | 0.74 | 0.67 | 0.74 | 44.0 |
| 32 | R2 | 365 | 1.4 | 0.843 | 45.3 | LOS D | 16.4 | 116.0 | 1.00 | 0.89 | 1.11 | 36.8 |
| Approach |  | 1563 | 2.1 | 0.843 | 30.2 | LOS C | 33.9 | 241.1 | 0.74 | 0.71 | 0.76 | 43.1 |
| All Ve | icles | 3880 | 2.3 | 1.024 | 64.4 | LOS E | 58.8 | 421.3 | 0.78 | 0.89 | 1.02 | 28.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^11]
## MOVEMENT SUMMARY

siof Site: 104 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road)-FRI AM 2028 Base]

审审 Network: N101 [Pacific Hwy-Wentworth Ave-FRI AM 2028 Base]
Staged crossing Stage 1 (Minor Road) at three-way intersection with 5 -lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn | Demand Flows Arrival Flows |  |  |  | Deg. <br> Satn <br> v/c | Average Delay sec | Level of Service | 95\% Back of Queue Vehicles Distance veh |  | Prop. Queued | Effective Stop Rate | Aver. Averag No. <br> Cycles Speed <br> km/h |  |
|  | Total veh/h |  |  | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ |  |  |  |  |  |  |  |  |  |
| South: Minor Road (Stage 1) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L 2 | 31 | 3.4 | 31 | 3.4 | 0.077 | 15.6 | LOS B | 0.3 | 1.8 | 0.68 | 1.00 | 0.68 | 44.2 |
| 2 T1 | 4 | 0.0 | 4 | 0.0 | 0.070 | 64.8 | LOS E | 0.2 | 1.4 | 0.95 | 1.00 | 0.95 | 19.7 |
| Approach | 35 | 3.0 | 35 | 3.0 | 0.077 | 21.6 | LOS B | 0.3 | 1.8 | 0.72 | 1.00 | 0.72 | 39.9 |
| East: Major Road East |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 7 | 14.3 | 7 | 14.3 | 0.406 | 5.8 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 57.5 |
| $5 \quad \mathrm{~T} 1$ | 1544 | 2.9 | 1544 | 2.9 | 0.406 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| 6 R2 | 1 | 100.0 | 1 | $\begin{array}{r} 100 . \\ 0 \end{array}$ | 0.001 | 6.4 | LOSA | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 50.6 |
| Approach | 1553 | 3.0 | 1553 | 3.0 | 0.406 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.8 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 12 R 2 | 7 | 0.0 | 7 | 0.0 | 0.066 | 27.7 | LOS B | 0.2 | 1.2 | 0.90 | 0.96 | 0.90 | 36.3 |
| 12u U | 3 | 0.0 | 3 | 0.0 | 0.066 | 28.5 | LOS C | 0.2 | 1.2 | 0.90 | 0.96 | 0.90 | 29.8 |
| Approach | 11 | 0.0 | 11 | 0.0 | 0.066 | 27.9 | NA | 0.2 | 1.2 | 0.90 | 0.96 | 0.90 | 34.6 |
| All Vehicles | 1598 | 3.0 | 1598 | 3.0 | 0.406 | 0.7 | NA | 0.3 | 1.8 | 0.02 | 0.03 | 0.02 | 59.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: X:117395 Doyalson Wyee RSLl07 Modelling Files\190614\17395_190617_Sc3 2028 Base.sip8

## MOVEMENT SUMMARY

$\nabla$ Site: 105 [Pacific Hwy-Wentworth Ave-Stage 2 (Median) NSW-FRI AM 2028 Base ]

审审 Network: N101 [Pacific Hwy-Wentworth Ave-FRI AM 2028 Base]
Staged crossing Stage 2 (Median) at three-way intersection with 5 -lane major road
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand Flows Arrival Flows |  |  |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Queue Vehicles Distance veh |  | Prop. Queued | Effective Stop Rate | Aver. Averag No. e Cycles Speed km/h |  |
| South: Median Storage Area |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 R2 | 4 | 0.0 | 4 | 0.0 | 0.008 | 4.8 | LOS A | 0.0 | 0.1 | 0.63 | 0.53 | 0.63 | 46.6 |
| Approach | 4 | 0.0 | 4 | 0.0 | 0.008 | 4.8 | LOS A | 0.0 | 0.1 | 0.63 | 0.53 | 0.63 | 46.6 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 T1 | 1107 | 8.8 | 1107 | 8.8 | 0.300 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| Approach | 1107 | 8.8 | 1107 | 8.8 | 0.300 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| All Vehicles | 1112 | 8.8 | 1112 | 8.8 | 0.300 | 0.0 | NA | 0.0 | 0.1 | 0.00 | 0.00 | 0.00 | 59.9 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

STITF Site: 106 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road)-FRI PM 2028 Base ]

审禹 Network: N101 [Pacific Hwy-Wentworth Ave-FRI PM 2028 Base]
Staged crossing Stage 1 (Minor Road) at three-way intersection with 5 -lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn | Demand Flows Arrival Flows |  |  |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Queue Vehicles Distance veh |  | Prop. Queued | Effective Stop Rate | Aver. Averag No. <br> Cycles Speed km/h |  |
|  | Total veh/h |  |  | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ |  |  |  |  |  |  |  |  |  |
| South: Minor Road (Stage 1) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L 2 | 28 | 3.7 | 28 | 3.7 | 0.050 | 12.4 | LOSA | 0.2 | 1.2 | 0.54 | 0.95 | 0.54 | 46.3 |
| 2 T1 | 3 | 0.0 | 3 | 0.0 | 0.021 | 30.0 | LOS C | 0.1 | 0.5 | 0.86 | 1.00 | 0.86 | 31.3 |
| Approach | 32 | 3.3 | 32 | 3.3 | 0.050 | 14.1 | LOS A | 0.2 | 1.2 | 0.57 | 0.96 | 0.57 | 44.8 |
| East: Major Road East |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 8 | 0.0 | 8 | 0.0 | 0.303 | 5.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 58.2 |
| $5 \quad \mathrm{~T} 1$ | 1146 | 3.4 | 1146 | 3.4 | 0.303 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| 6 R2 | 1 | 0.0 | 1 | 0.0 | 0.001 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 50.6 |
| Approach | 1156 | 3.4 | 1156 | 3.4 | 0.303 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 12 R 2 | 28 | 7.4 | 28 | 7.4 | 0.107 | 16.8 | LOS B | 0.3 | 2.4 | 0.80 | 0.92 | 0.80 | 42.0 |
| 12u U | 3 | 33.3 | 3 | 33.3 | 0.107 | 22.9 | LOS B | 0.3 | 2.4 | 0.80 | 0.92 | 0.80 | 32.8 |
| Approach | 32 | 10.0 | 32 | 10.0 | 0.107 | 17.5 | NA | 0.3 | 2.4 | 0.80 | 0.92 | 0.80 | 41.2 |
| All Vehicles | 1219 | 3.5 | 1219 | 3.5 | 0.303 | 0.9 | NA | 0.3 | 2.4 | 0.04 | 0.05 | 0.04 | 58.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

$\nabla$ Site: 107 [Pacific Hwy-Wentworth Ave-Stage 2 (Median) NSW- FRI PM 2028 Base ]

审禹 Network: N101 [Pacific Hwy-Wentworth Ave-FRI PM 2028 Base]
Staged crossing Stage 2 (Median) at three-way intersection with 5 -lane major road.
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)


Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

siof Site: 108 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road)-SAT 2028 Base]

审审 Network: N101 [Pacific Hwy-Wentworth Ave-SAT 2028

Staged crossing Stage 1 (Minor Road) at three-way intersection with 5-lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand <br> Total veh/h | ows | Arriva <br> Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay | Level of Service | 95\% <br> Vehicles veh | Back of ue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Averag <br> Speed <br> km/h |
| South: Minor Road (Stage 1) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 37 | 1.0 | 37 | 1.0 | 0.066 | 12.5 | LOS A | 0.2 | 1.6 | 0.55 | 0.97 | 0.55 | 46.5 |
| 2 T1 | 11 | 0.0 | 11 | 0.0 | 0.076 | 33.0 | LOS C | 0.2 | 1.6 | 0.88 | 1.00 | 0.88 | 29.8 |
| Approach | 47 | 0.7 | 47 | 0.7 | 0.076 | 17.1 | LOS B | 0.2 | 1.6 | 0.62 | 0.98 | 0.62 | 42.6 |
| East: Major Road East |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 11 | 0.0 | 11 | 0.0 | 0.316 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 58.2 |
| $5 \quad \mathrm{~T} 1$ | 1199 | 3.0 | 1199 | 3.0 | 0.316 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 59.9 |
| 6 R2 | 1 | 0.0 | 1 | 0.0 | 0.001 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 50.6 |
| Approach | 1211 | 3.0 | 1211 | 3.0 | 0.316 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 59.8 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 12 R 2 | 14 | 0.0 | 14 | 0.0 | 0.048 | 16.4 | LOS B | 0.1 | 1.0 | 0.79 | 0.91 | 0.79 | 42.8 |
| 12u U | 1 | 0.0 | 1 | 0.0 | 0.048 | 17.4 | LOS B | 0.1 | 1.0 | 0.79 | 0.91 | 0.79 | 36.8 |
| Approach | 15 | 0.0 | 15 | 0.0 | 0.048 | 16.5 | NA | 0.1 | 1.0 | 0.79 | 0.91 | 0.79 | 42.5 |
| All Vehicles | 1273 | 2.9 | 1273 | 2.9 | 0.316 | 0.9 | NA | 0.2 | 1.6 | 0.03 | 0.05 | 0.03 | 58.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

$\nabla$ Site: 109 [Pacific Hwy-Wentworth Ave-Stage 2 (Median)
NSW- SAT 2028 Base]
审审 Network: N101 [Pacific Hwy-Wentworth Ave-SAT 2028

Base]
Staged crossing Stage 2 (Median) at three-way intersection with 5 -lane major road.
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)


Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

## Site: 110 [Pacific Hwy-Rutley Rd -FRI AM 2028 Base]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 102 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Deman Total veh/h | $\begin{gathered} \text { Flows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Pacific Hwy - E |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 1428 | 3.5 | 0.375 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.8 |
| 6 | R2 | 220 | 5.7 | 0.404 | 25.8 | LOS B | 8.0 | 58.5 | 0.80 | 0.82 | 0.80 | 48.0 |
| Appr |  | 1648 | 3.8 | 0.404 | 3.5 | LOS A | 8.0 | 58.5 | 0.11 | 0.11 | 0.11 | 73.3 |
| North: Rutley Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 221 | 6.2 | 0.124 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 64.3 |
| 9 | R2 | 246 | 6.8 | 0.788 | 55.3 | LOS D | 12.8 | 94.8 | 1.00 | 0.89 | 1.16 | 34.5 |
| Appr |  | 467 | 6.5 | 0.788 | 32.8 | LOS C | 12.8 | 94.8 | 0.53 | 0.75 | 0.61 | 44.3 |
| West: Pacific Hwy - W |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 125 | 10.1 | 0.123 | 12.2 | LOS A | 1.9 | 14.2 | 0.38 | 0.69 | 0.38 | 57.8 |
| 11 | T1 | 879 | 6.3 | 0.520 | 21.5 | LOS B | 15.6 | 115.2 | 0.77 | 0.68 | 0.77 | 54.4 |
| Approach |  | 1004 | 6.8 | 0.520 | 20.3 | LOS B | 15.6 | 115.2 | 0.72 | 0.68 | 0.72 | 54.8 |
| All Ve | icles | 3120 | 5.2 | 0.788 | 13.3 | LOS A | 15.6 | 115.2 | 0.37 | 0.39 | 0.38 | 60.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

## Site: 111 [Pacific Hwy-Rutley Rd -FRI PM 2028 Base]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 102 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | Turn | Deman <br> Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. <br> Satn <br> V/C | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Pacific Hwy - E 0.0 0.0 |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 1040 | 2.5 | 0.271 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| 6 | R2 | 253 | 1.7 | 0.462 | 37.6 | LOS C | 9.9 | 70.2 | 0.86 | 0.91 | 1.18 | 42.2 |
| Appr | ach | 1293 | 2.4 | 0.462 | 7.4 | LOS A | 9.9 | 70.2 | 0.17 | 0.18 | 0.23 | 68.0 |
| North: Rutley Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 374 | 0.0 | 0.201 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 66.2 |
| 9 | R2 | 245 | 2.1 | 0.977 | 87.1 | LOS F | 16.9 | 120.5 | 1.00 | 1.06 | 1.71 | 26.8 |
| Approach |  | 619 | 0.9 | 0.977 | 39.1 | LOS C | 16.9 | 120.5 | 0.40 | 0.79 | 0.68 | 41.9 |
| West: Pacific Hwy - W |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 271 | 2.7 | 0.232 | 10.5 | LOSA | 2.9 | 20.9 | 0.38 | 0.70 | 0.38 | 61.4 |
| 11 | T1 | 1399 | 1.4 | 0.821 | 30.0 | LOS C | 32.4 | 229.6 | 0.94 | 0.89 | 1.00 | 48.2 |
| Appr | ach | 1669 | 1.6 | 0.821 | 26.9 | LOS B | 32.4 | 229.6 | 0.85 | 0.86 | 0.90 | 50.0 |
| All V | hicles | 3581 | 1.8 | 0.977 | 21.9 | LOS B | 32.4 | 229.6 | 0.53 | 0.60 | 0.62 | 53.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

## Site: 112 [Pacific Hwy-Rutley Rd -SAT 2028 Base]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 102 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand <br> Total veh/h |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Pacific Hwy - E |  |  |  |  |  |  |  |  |  |  |  |
| $5 \quad$ T1 | 1066 | 1.4 | 0.276 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| 6 R2 | 203 | 2.6 | 0.343 | 26.8 | LOS B | 7.6 | 54.4 | 0.80 | 0.81 | 0.80 | 48.0 |
| Approach | 1269 | 1.6 | 0.343 | 4.3 | LOS A | 7.6 | 54.4 | 0.13 | 0.13 | 0.13 | 72.2 |
| North: Rutley Rd |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 215 | 1.5 | 0.117 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 65.8 |
| 9 R2 | 187 | 1.7 | 0.744 | 56.4 | LOS D | 9.6 | 68.4 | 1.00 | 0.86 | 1.13 | 34.6 |
| Approach | 402 | 1.6 | 0.744 | 30.3 | LOS C | 9.6 | 68.4 | 0.47 | 0.72 | 0.53 | 46.4 |
| West: Pacific Hwy - W |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 206 | 1.0 | 0.189 | 12.2 | LOS A | 3.2 | 22.5 | 0.40 | 0.70 | 0.40 | 60.0 |
| 11 T1 | 1093 | 1.9 | 0.643 | 23.8 | LOS B | 21.1 | 150.3 | 0.84 | 0.75 | 0.84 | 52.5 |
| Approach | 1299 | 1.8 | 0.643 | 22.0 | LOS B | 21.1 | 150.3 | 0.77 | 0.74 | 0.77 | 53.6 |
| All Vehicles | 2971 | 1.7 | 0.744 | 15.6 | LOS B | 21.1 | 150.3 | 0.45 | 0.48 | 0.46 | 58.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: X:\17395 Doyalson Wyee RSLl07 Modelling Files\190614\17395_190617_Sc3 2028 Base.sip8

## MOVEMENT SUMMARY

## Site: 101 [Pacific HIghway-Wyee Road-Scenic Drive -FRI AM 2028+Stage 1]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 104 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{array}{\|l\|} \hline \text { Mov } \\ \text { ID } \\ \hline \end{array}$ |  | Demand Total veh/h | $\begin{gathered} \text { Flows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 482 | 1.6 | 0.263 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.9 |
| 22 | T1 | 161 | 8.2 | 0.944 | 71.1 | LOS F | 10.2 | 76.4 | 0.99 | 1.11 | 1.67 | 27.9 |
| 23 | R2 | 234 | 3.3 | 1.043 | 123.7 | LOS F | 20.1 | 145.0 | 1.00 | 1.28 | 2.04 | 16.0 |
| Appr | ach | 877 | 3.2 | 1.043 | 49.1 | LOS D | 20.1 | 145.0 | 0.45 | 0.84 | 0.85 | 32.2 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 123 | 4.9 | 0.123 | 13.0 | LOS A | 1.6 | 11.7 | 0.50 | 0.72 | 0.50 | 48.2 |
| 25 | T1 | 1681 | 2.7 | 1.106 | 156.1 | LOS F | 92.4 | 662.1 | 1.00 | 1.69 | 2.16 | 14.1 |
| 26 | R2 | 45 | 0.0 | 0.507 | 63.6 | LOS E | 2.4 | 17.1 | 1.00 | 0.74 | 1.02 | 25.5 |
| Appr |  | 1849 | 2.8 | 1.106 | 144.3 | LOS F | 92.4 | 662.1 | 0.97 | 1.60 | 2.02 | 15.0 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 59 | 4.1 | 0.872 | 65.5 | LOS E | 10.7 | 79.5 | 1.00 | 1.05 | 1.75 | 25.7 |
| 28 | T1 | 139 | 7.8 | 0.872 | 59.8 | LOSE | 10.7 | 79.5 | 1.00 | 1.05 | 1.75 | 30.0 |
| 29 | R2 | 204 | 4.1 | 1.085 | 152.5 | LOS F | 19.8 | 143.1 | 1.00 | 1.39 | 2.28 | 17.2 |
| Appr | ach | 402 | 5.4 | 1.085 | 107.7 | LOS F | 19.8 | 143.1 | 1.00 | 1.23 | 2.02 | 21.2 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 125 | 14.9 | 0.075 | 7.8 | LOS A | 0.0 | 0.0 | 0.00 | 0.59 | 0.00 | 61.8 |
| 31 | T1 | 1135 | 7.9 | 0.644 | 21.6 | LOS B | 21.8 | 162.8 | 0.81 | 0.72 | 0.81 | 48.9 |
| 32 | R2 | 215 | 8.7 | 0.912 | 71.0 | LOS F | 13.1 | 98.6 | 1.00 | 0.99 | 1.47 | 29.2 |
| Approach |  | 1475 | 8.6 | 0.912 | 27.6 | LOS B | 21.8 | 162.8 | 0.77 | 0.75 | 0.83 | 44.5 |
| All V | icles | 4603 | 5.0 | 1.106 | 85.6 | LOS F | 92.4 | 662.1 | 0.81 | 1.15 | 1.42 | 23.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^12]
## MOVEMENT SUMMARY

## Site: 102 [Pacific HIghway-Wyee Road-Scenic Drive -FRI PM 2028+Stage 1]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 184 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 264 | 3.2 | 0.145 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.8 |
| 22 | T1 | 177 | 2.1 | 1.079 | 187.3 | LOS F | 24.8 | 176.6 | 1.00 | 1.27 | 1.80 | 14.5 |
| 23 | R2 | 259 | 1.8 | 1.251 | 322.8 | LOS F | 47.7 | 339.0 | 1.00 | 1.38 | 2.24 | 7.1 |
| Appr | ach | 700 | 2.4 | 1.251 | 168.8 | LOS F | 47.7 | 339.0 | 0.62 | 1.03 | 1.28 | 14.4 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 236 | 2.1 | 0.304 | 26.9 | LOS B | 8.7 | 61.7 | 0.70 | 0.78 | 0.70 | 38.8 |
| 25 | T1 | 1147 | 3.7 | 1.240 | 301.3 | LOS F | 111.0 | 801.8 | 1.00 | 1.68 | 2.13 | 7.8 |
| 26 | R2 | 68 | 0.0 | 0.452 | 95.8 | LOS F | 6.0 | 42.0 | 1.00 | 0.77 | 1.00 | 19.6 |
| Appr | ach | 1452 | 3.3 | 1.240 | 247.0 | LOS F | 111.0 | 801.8 | 0.95 | 1.49 | 1.85 | 9.3 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 49 | 2.3 | 0.982 | 157.1 | LOS F | 31.6 | 223.9 | 1.00 | 1.21 | 2.10 | 13.9 |
| 28 | T1 | 211 | 1.1 | 0.982 | 151.5 | LOS F | 31.6 | 223.9 | 1.00 | 1.21 | 2.10 | 17.2 |
| 29 | R2 | 180 | 0.6 | 1.114 | 216.6 | LOS F | 26.7 | 188.2 | 1.00 | 1.22 | 1.89 | 13.0 |
| Appr | ach | 440 | 1.0 | 1.114 | 178.8 | LOS F | 31.6 | 223.9 | 1.00 | 1.21 | 2.02 | 14.9 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 145 | 2.6 | 0.080 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 65.4 |
| 31 | T1 | 1757 | 1.8 | 1.139 | 206.1 | LOS F | 163.4 | 1161.4 | 1.00 | 1.54 | 1.78 | 11.1 |
| 32 | R2 | 561 | 1.1 | 1.158 | 210.9 | LOS F | 68.7 | 485.4 | 1.00 | 1.20 | 1.88 | 12.0 |
| Approach |  | 2463 | 1.7 | 1.158 | 195.5 | LOS F | 163.4 | 1161.4 | 0.94 | 1.41 | 1.70 | 12.1 |
| All Vehicles |  | 5055 | 2.2 | 1.251 | 205.1 | LOS F | 163.4 | 1161.4 | 0.91 | 1.36 | 1.71 | 11.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^13]
## MOVEMENT SUMMARY

## Site: 103 [Pacific HIghway-Wyee Road-Scenic Drive -SAT 2028+Stage 1]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 157 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 353 | 1.0 | 0.191 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.9 |
| 22 | T1 | 156 | 3.1 | 0.963 | 103.7 | LOS F | 14.4 | 103.3 | 0.96 | 1.07 | 1.52 | 22.4 |
| 23 | R2 | 253 | 1.9 | 1.086 | 180.9 | LOS F | 32.2 | 228.8 | 1.00 | 1.22 | 1.88 | 11.8 |
| Appr |  | 761 | 1.7 | 1.086 | 83.9 | LOS F | 32.2 | 228.8 | 0.53 | 0.87 | 0.93 | 23.8 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 209 | 1.2 | 0.235 | 20.3 | LOS B | 5.7 | 40.4 | 0.62 | 0.76 | 0.62 | 42.8 |
| 25 | T1 | 1171 | 2.4 | 1.057 | 144.4 | LOS F | 74.4 | 531.7 | 1.00 | 1.34 | 1.64 | 15.0 |
| 26 | R2 | 56 | 4.5 | 0.695 | 93.3 | LOS F | 4.6 | 33.1 | 1.00 | 0.80 | 1.15 | 19.9 |
| Appr | ach | 1436 | 2.3 | 1.057 | 124.3 | LOS F | 74.4 | 531.7 | 0.94 | 1.23 | 1.47 | 16.8 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 72 | 3.2 | 0.932 | 105.5 | LOS F | 18.3 | 131.5 | 1.00 | 1.13 | 1.84 | 18.7 |
| 28 | T1 | 140 | 3.3 | 0.932 | 99.9 | LOS F | 18.3 | 131.5 | 1.00 | 1.13 | 1.84 | 22.6 |
| 29 | R2 | 151 | 2.3 | 0.964 | 110.8 | LOS F | 14.1 | 100.5 | 1.00 | 1.05 | 1.55 | 21.9 |
| Appr | ach | 362 | 2.8 | 0.964 | 105.6 | LOS F | 18.3 | 131.5 | 1.00 | 1.09 | 1.72 | 21.6 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 138 | 5.5 | 0.077 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 64.5 |
| 31 | T1 | 1219 | 1.7 | 0.717 | 28.2 | LOS B | 41.4 | 293.7 | 0.77 | 0.70 | 0.77 | 43.7 |
| 32 | R2 | 365 | 1.4 | 0.945 | 71.3 | LOS F | 22.3 | 157.7 | 1.00 | 0.99 | 1.39 | 29.2 |
| Approach |  | 1722 | 1.9 | 0.945 | 35.7 | LOS C | 41.4 | 293.7 | 0.76 | 0.75 | 0.84 | 39.8 |
| All Vehicles |  | 4281 | 2.1 | 1.086 | 79.9 | LOS F | 74.4 | 531.7 | 0.80 | 0.96 | 1.14 | 24.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^14]
## MOVEMENT SUMMARY

## Site: 110 [Pacific Hwy-Rutley Rd -FRI AM 2028+Stage 1]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 102 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Deman Total veh/h | $\begin{array}{r} =\text { lows } \\ \text { HV } \\ \% \end{array}$ | Deg. Satn v/C | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Pacific Hwy - E |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 1509 | 3.3 | 0.395 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.8 |
| 6 | R2 | 220 | 5.7 | 0.424 | 28.3 | LOS B | 8.7 | 63.7 | 0.85 | 0.83 | 0.85 | 46.5 |
| Appr | ach | 1729 | 3.7 | 0.424 | 3.6 | LOS A | 8.7 | 63.7 | 0.11 | 0.11 | 0.11 | 73.1 |
| North: Rutley Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 221 | 6.2 | 0.124 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 64.3 |
| 9 | R2 | 246 | 6.8 | 0.788 | 55.3 | LOS D | 12.8 | 94.8 | 1.00 | 0.89 | 1.16 | 34.5 |
| Appr | ach | 467 | 6.5 | 0.788 | 32.8 | LOS C | 12.8 | 94.8 | 0.53 | 0.75 | 0.61 | 44.3 |
| West: Pacific Hwy - W |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 125 | 10.1 | 0.127 | 13.2 | LOS A | 2.1 | 15.7 | 0.41 | 0.69 | 0.41 | 56.9 |
| 11 | T1 | 972 | 5.7 | 0.573 | 22.2 | LOS B | 17.8 | 130.8 | 0.79 | 0.70 | 0.79 | 53.8 |
| Approach |  | 1097 | 6.2 | 0.573 | 21.2 | LOS B | 17.8 | 130.8 | 0.75 | 0.70 | 0.75 | 54.1 |
| All Vehicles |  | 3294 | 4.9 | 0.788 | 13.6 | LOS A | 17.8 | 130.8 | 0.38 | 0.40 | 0.39 | 60.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

## Site: 111 [Pacific Hwy-Rutley Rd -FRI PM 2028+Stage 1]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 102 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand <br> Total veh/h |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Pacific Hwy - E |  |  |  |  |  |  |  |  |  |  |  |
| $5 \quad$ T1 | 1134 | 2.3 | 0.295 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| 6 R2 | 253 | 1.7 | 0.470 | 39.1 | LOS C | 9.9 | 70.6 | 0.87 | 0.91 | 1.22 | 41.4 |
| Approach | 1386 | 2.2 | 0.470 | 7.2 | LOS A | 9.9 | 70.6 | 0.16 | 0.17 | 0.22 | 68.3 |
| North: Rutley Rd |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 374 | 0.0 | 0.201 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 66.2 |
| 9 R2 | 245 | 2.1 | 0.977 | 87.1 | LOS F | 16.9 | 120.5 | 1.00 | 1.06 | 1.71 | 26.8 |
| Approach | 619 | 0.9 | 0.977 | 39.1 | LOS C | 16.9 | 120.5 | 0.40 | 0.79 | 0.68 | 41.9 |
| West: Pacific Hwy - W |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 271 | 2.7 | 0.223 | 9.9 | LOS A | 2.5 | 17.6 | 0.35 | 0.70 | 0.35 | 62.1 |
| 11 T1 | 1487 | 1.3 | 0.872 | 36.0 | LOS C | 38.5 | 272.5 | 0.98 | 0.98 | 1.11 | 44.7 |
| Approach | 1758 | 1.6 | 0.872 | 32.0 | LOS C | 38.5 | 272.5 | 0.88 | 0.94 | 0.99 | 46.7 |
| All Vehicles | 3763 | 1.7 | 0.977 | 24.0 | LOS B | 38.5 | 272.5 | 0.54 | 0.63 | 0.66 | 51.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

## Site: 112 [Pacific Hwy-Rutley Rd -SAT 2028+Stage 1]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 102 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/C | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Pacific Hwy - E |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 1135 | 1.3 | 0.293 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| 6 | R2 | 203 | 2.6 | 0.351 | 28.9 | LOS C | 7.7 | 55.4 | 0.82 | 0.83 | 0.86 | 46.7 |
| Appr |  | 1338 | 1.5 | 0.351 | 4.4 | LOS A | 7.7 | 55.4 | 0.12 | 0.13 | 0.13 | 72.1 |
| North: Rutley Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 215 | 1.5 | 0.117 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 65.8 |
| 9 | R2 | 187 | 1.7 | 0.744 | 56.4 | LOS D | 9.6 | 68.4 | 1.00 | 0.86 | 1.13 | 34.6 |
| Appr | ach | 402 | 1.6 | 0.744 | 30.3 | LOS C | 9.6 | 68.4 | 0.47 | 0.72 | 0.53 | 46.4 |
| West: Pacific Hwy - W |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 206 | 1.0 | 0.188 | 12.0 | LOS A | 3.1 | 21.6 | 0.40 | 0.70 | 0.40 | 60.3 |
| 11 | T1 | 1158 | 1.8 | 0.681 | 24.4 | LOS B | 22.9 | 163.0 | 0.86 | 0.77 | 0.86 | 52.1 |
| Approach |  | 1364 | 1.7 | 0.681 | 22.5 | LOS B | 22.9 | 163.0 | 0.79 | 0.76 | 0.79 | 53.2 |
| All Ve | icles | 3104 | 1.6 | 0.744 | 15.7 | LOS B | 22.9 | 163.0 | 0.46 | 0.48 | 0.47 | 58.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

siof Site: 104 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road)-FRI AM 2028+Stage 1]

审官 Network: N101 [Pacific Hwy-Wentworth Ave-FRI AM 2028+Stage 1]
Staged crossing Stage 1 (Minor Road) at three-way intersection with 5-lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)


Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: TTPP - THE TRANSPORT PLANNING PARTNERSHIP | Processed: Friday, 7 December 2018 12:15:29 PM
Project: X:\17395 Doyalson Wyee RSLl07 Modelling Files\190614\17395_190617 Sc4 2028 +Stage 1.sip8

## MOVEMENT SUMMARY

$\nabla$ Site: 105 [Pacific Hwy-Wentworth Ave-Stage 2 (Median) NSW-FRI AM 2028+Stage 1]

蚛审 Network: N101 [Pacific Hwy-Wentworth Ave-FRI AM 2028+Stage 1]
Staged crossing Stage 2 (Median) at three-way intersection with 5 -lane major road.
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)


Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

STITF Site: 106 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road)-FRI PM 2028+Stage 1]

审禹 Network: N101 [Pacific Hwy-Wentworth Ave-FRI PM 2028+Stage 1]
Staged crossing Stage 1 (Minor Road) at three-way intersection with 5-lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand <br> Total veh/h | Flows | Arrival <br> Total veh/h | $\begin{gathered} \text { Flows } \\ \mathrm{HV} \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service |  | Back of eue Distance | Prop. Queued | Effective Stop Rate |  | Averag Speed km/h |
| South: Minor Road (Stage 1) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 39 | 2.7 | 39 | 2.7 | 0.076 | 13.5 | LOS A | 0.3 | 2.0 | 0.59 | 0.98 | 0.59 | 45.6 |
| $2 \quad \mathrm{~T} 1$ | 6 | 0.0 | 6 | 0.0 | 0.055 | 38.2 | LOS C | 0.2 | 1.2 | 0.90 | 1.00 | 0.90 | 27.5 |
| Approach | 45 | 2.3 | 45 | 2.3 | 0.076 | 17.0 | LOS B | 0.3 | 2.0 | 0.63 | 0.98 | 0.63 | 42.8 |
| East: Major Road East |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 22 | 0.0 | 22 | 0.0 | 0.371 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 57.9 |
| $5 \quad \mathrm{~T} 1$ | 1399 | 2.8 | 1399 | 2.8 | 0.371 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 59.8 |
| 6 R2 | 1 | 0.0 | 1 | 0.0 | 0.001 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 48.2 |
| Approach | 1422 | 2.7 | 1422 | 2.7 | 0.371 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 59.7 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 12 R 2 | 69 | 3.0 | 69 | 3.0 | 0.285 | 21.6 | LOS B | 0.9 | 6.7 | 0.86 | 0.97 | 0.97 | 39.4 |
| 12u U | 3 | 33.3 | 3 | 33.3 | 0.285 | 30.1 | LOS C | 0.9 | 6.7 | 0.86 | 0.97 | 0.97 | 30.2 |
| Approach | 73 | 4.3 | 73 | 4.3 | 0.285 | 22.0 | NA | 0.9 | 6.7 | 0.86 | 0.97 | 0.97 | 39.0 |
| All Vehicles | 1540 | 2.8 | 1540 | 2.8 | 0.371 | 1.7 | NA | 0.9 | 6.7 | 0.06 | 0.08 | 0.06 | 57.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

$\nabla$ Site: 107 [Pacific Hwy-Wentworth Ave-Stage 2 (Median) NSW-FRI PM 2028+Stage 1]

㿾审 Network: N101 [Pacific Hwy-Wentworth Ave-FRI PM 2028+Stage 1]
Staged crossing Stage 2 (Median) at three-way intersection with 5 -lane major road
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand Flows Arrival Flows |  |  |  | Deg. Satn <br> v/c | Average Delay sec | Level of Service | 95\% Back of Queue Vehicles Distance veh |  | Prop. Queued | Effective Stop Rate | Aver. Averag No. e Cycles Speed km/h |  |
| South: Median Storage Area |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 R2 | 6 | 0.0 | 6 | 0.0 | 0.039 | 19.6 | LOS B | 0.1 | 0.6 | 0.89 | 0.89 | 0.89 | 30.1 |
| Approach | 6 | 0.0 | 6 | 0.0 | 0.039 | 19.6 | LOS B | 0.1 | 0.6 | 0.89 | 0.89 | 0.89 | 30.1 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 T1 | 1952 | 2.0 | 1952 | 2.0 | 0.507 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.8 |
| Approach | 1952 | 2.0 | 1952 | 2.0 | 0.507 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.8 |
| All Vehicles | 1958 | 2.0 | 1958 | 2.0 | 0.507 | 0.1 | NA | 0.1 | 0.6 | 0.00 | 0.00 | 0.00 | 59.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

siof Site: 108 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road)-SAT 2028+Stage 1]

审审 Network: N101 [Pacific Hwy-Wentworth Ave-SAT 2028 +Stage 1]
Staged crossing Stage 1 (Minor Road) at three-way intersection with 5-lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)


Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

V Site: 109 [Pacific Hwy-Wentworth Ave-Stage 2 (Median) NSW- SAT 2028+Stage 1]

审审 Network: N101 [Pacific Hwy-Wentworth Ave-SAT 2028 +Stage 1]
Staged crossing Stage 2 (Median) at three-way intersection with 5-lane major road.
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)


Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

## Site: 101 [Pacific HIghway-Wyee Road-Scenic Drive -FRI AM 2038 Base]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 104 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand <br> Total <br> veh/h | $\begin{gathered} \text { =lows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 519 | 1.6 | 0.283 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.8 |
| 22 | T1 | 174 | 8.2 | 0.992 | 90.1 | LOS F | 12.5 | 93.7 | 0.98 | 1.21 | 1.86 | 24.4 |
| 23 | R2 | 203 | 4.1 | 0.823 | 57.9 | LOS E | 11.0 | 79.7 | 1.00 | 0.93 | 1.25 | 26.5 |
| Appro |  | 896 | 3.4 | 0.992 | 33.9 | LOS C | 12.5 | 93.7 | 0.42 | 0.75 | 0.65 | 37.9 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 117 | 5.7 | 0.129 | 14.4 | LOS A | 1.8 | 12.9 | 0.55 | 0.73 | 0.55 | 47.0 |
| 25 | T1 | 1604 | 3.2 | 1.219 | 250.7 | LOS F | 111.7 | 803.2 | 1.00 | 2.06 | 2.80 | 9.3 |
| 26 | R2 | 43 | 0.0 | 0.483 | 63.4 | LOS E | 2.3 | 16.3 | 1.00 | 0.73 | 1.00 | 25.5 |
| Appro | ch | 1764 | 3.3 | 1.219 | 230.5 | LOS F | 111.7 | 803.2 | 0.97 | 1.94 | 2.61 | 10.0 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 52 | 5.0 | 0.682 | 56.0 | LOS D | 9.5 | 70.3 | 0.99 | 0.91 | 1.48 | 28.3 |
| 28 | T1 | 149 | 7.8 | 0.682 | 50.3 | LOS D | 9.5 | 70.3 | 0.99 | 0.91 | 1.48 | 32.6 |
| 29 | R2 | 218 | 4.1 | 0.945 | 75.9 | LOS F | 14.0 | 101.5 | 1.00 | 1.09 | 1.60 | 27.5 |
| Appr |  | 419 | 5.5 | 0.945 | 64.3 | LOS E | 14.0 | 101.5 | 1.00 | 1.01 | 1.54 | 29.3 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 158 | 14.9 | 0.094 | 7.8 | LOS A | 0.0 | 0.0 | 0.00 | 0.59 | 0.00 | 61.8 |
| 31 | T1 | 1193 | 9.5 | 0.820 | 31.1 | LOS C | 31.1 | 235.4 | 0.91 | 0.87 | 0.99 | 41.8 |
| 32 | R2 | 272 | 8.7 | 1.077 | 147.7 | LOS F | 25.9 | 194.5 | 1.00 | 1.26 | 2.18 | 17.8 |
| Approach |  | 1622 | 9.9 | 1.077 | 48.4 | LOS D | 31.1 | 235.4 | 0.84 | 0.91 | 1.09 | 33.7 |
| All Ve | icles | 4701 | 5.8 | 1.219 | 115.4 | LOS F | 111.7 | 803.2 | 0.82 | 1.27 | 1.62 | 18.9 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^15]
## MOVEMENT SUMMARY

## Site: 102 [Pacific HIghway-Wyee Road-Scenic Drive -FRI PM 2038 Base]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 184 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Deman <br> Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 297 | 3.2 | 0.163 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.8 |
| 22 | T1 | 198 | 2.1 | 1.209 | 285.5 | LOS F | 34.5 | 245.8 | 1.00 | 1.48 | 2.16 | 10.2 |
| 23 | R2 | 232 | 2.3 | 1.179 | 264.8 | LOS F | 38.5 | 274.6 | 1.00 | 1.29 | 2.05 | 8.5 |
| Appro |  | 726 | 2.6 | 1.209 | 164.5 | LOS F | 38.5 | 274.6 | 0.59 | 1.03 | 1.24 | 14.8 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 259 | 2.3 | 0.334 | 27.2 | LOS B | 9.7 | 69.0 | 0.71 | 0.79 | 0.71 | 38.6 |
| 25 | T1 | 1077 | 4.7 | 1.187 | 258.2 | LOS F | 97.3 | 708.5 | 1.00 | 1.57 | 1.98 | 9.0 |
| 26 | R2 | 74 | 0.0 | 0.487 | 96.2 | LOS F | 6.5 | 45.4 | 1.00 | 0.77 | 1.00 | 19.5 |
| Appro | ch | 1409 | 4.0 | 1.187 | 207.3 | LOS F | 97.3 | 708.5 | 0.95 | 1.38 | 1.70 | 10.8 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 41 | 3.0 | 1.025 | 85.2 | LOS F | 32.4 | 229.8 | 1.00 | 0.99 | 1.55 | 12.4 |
| 28 | T1 | 229 | 1.1 | 1.025 | 79.5 | LOS F | 32.4 | 229.8 | 1.00 | 0.99 | 1.55 | 15.5 |
| 29 | R2 | 196 | 0.6 | 1.199 | 282.0 | LOS F | 33.5 | 235.6 | 1.00 | 1.33 | 2.13 | 10.4 |
| Appr |  | 466 | 1.1 | 1.199 | 165.0 | LOS F | 33.5 | 235.6 | 1.00 | 1.13 | 1.79 | 12.7 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 164 | 2.6 | 0.090 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 65.4 |
| 31 | T1 | 1746 | 2.0 | 1.160 | 223.4 | LOS F | 170.8 | 1216.8 | 1.00 | 1.59 | 1.85 | 10.3 |
| 32 | R2 | 636 | 1.1 | 1.307 | 338.2 | LOS F | 101.3 | 716.0 | 1.00 | 1.35 | 2.31 | 8.4 |
| Approach |  | 2546 | 1.9 | 1.307 | 238.1 | LOS F | 170.8 | 1216.8 | 0.94 | 1.47 | 1.84 | 10.3 |
| All Ve | icles | 5148 | 2.5 | 1.307 | 212.7 | LOS F | 170.8 | 1216.8 | 0.90 | 1.35 | 1.71 | 11.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^16]
## MOVEMENT SUMMARY

## Site: 103 [Pacific HIghway-Wyee Road-Scenic Drive -SAT 2038 Base]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 157 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 396 | 1.0 | 0.215 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.9 |
| 22 | T1 | 175 | 3.1 | 1.081 | 176.1 | LOS F | 22.2 | 159.8 | 1.00 | 1.31 | 1.93 | 15.2 |
| 23 | R2 | 244 | 2.2 | 1.083 | 179.2 | LOS F | 30.9 | 220.6 | 1.00 | 1.22 | 1.87 | 11.9 |
| Appro |  | 815 | 1.8 | 1.083 | 94.2 | LOS F | 30.9 | 220.6 | 0.51 | 0.90 | 0.97 | 22.2 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 217 | 1.4 | 0.244 | 20.3 | LOS B | 5.9 | 42.1 | 0.62 | 0.76 | 0.62 | 42.8 |
| 25 | T1 | 1198 | 2.8 | 1.085 | 165.0 | LOS F | 81.2 | 582.5 | 1.00 | 1.42 | 1.74 | 13.4 |
| 26 | R2 | 58 | 5.1 | 0.725 | 93.9 | LOS F | 4.8 | 34.7 | 1.00 | 0.81 | 1.18 | 19.8 |
| Appr |  | 1473 | 2.7 | 1.085 | 140.9 | LOS F | 81.2 | 582.5 | 0.94 | 1.30 | 1.56 | 15.1 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 67 | 3.7 | 0.976 | 121.0 | LOS F | 20.7 | 149.3 | 1.00 | 1.19 | 1.96 | 17.0 |
| 28 | T1 | 153 | 3.3 | 0.976 | 115.3 | LOS F | 20.7 | 149.3 | 1.00 | 1.19 | 1.96 | 20.7 |
| 29 | R2 | 164 | 2.3 | 1.062 | 165.7 | LOS F | 19.6 | 139.6 | 1.00 | 1.19 | 1.86 | 16.2 |
| Appr |  | 384 | 2.9 | 1.062 | 137.8 | LOS F | 20.7 | 149.3 | 1.00 | 1.19 | 1.92 | 17.9 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 156 | 5.5 | 0.087 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 64.5 |
| 31 | T1 | 1201 | 1.9 | 0.721 | 28.2 | LOS B | 41.5 | 295.4 | 0.77 | 0.70 | 0.77 | 43.8 |
| 32 | R2 | 414 | 1.4 | 1.070 | 135.4 | LOS F | 35.9 | 254.0 | 1.00 | 1.12 | 1.74 | 16.2 |
| Approach |  | 1771 | 2.1 | 1.070 | 51.4 | LOS D | 41.5 | 295.4 | 0.76 | 0.79 | 0.93 | 30.2 |
| All Ve | icles | 4442 | 2.3 | 1.085 | 96.4 | LOS F | 81.2 | 582.5 | 0.80 | 1.01 | 1.23 | 21.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^17]
## MOVEMENT SUMMARY

siof Site: 104 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road)-FRI AM 2038 Base]

审审 Network: N101 [Pacific Hwy-Wentworth Ave-FRI AM 2038 Base]
Staged crossing Stage 1 (Minor Road) at three-way intersection with 5 -lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand <br> Total veh/h | Flows <br> HV <br> \% | Arriva <br> Total veh/h | $\begin{gathered} \text { Flows } \\ \mathrm{HV} \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service |  | of <br> ance <br> m | Prop. Queued | Effective Stop Rate |  | Averag Speed $\mathrm{km} / \mathrm{h}$ |
| South: Minor Road (Stage 1) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 31 | 3.4 | 31 | 3.4 | 0.095 | 17.9 | LOS B | 0.3 | 2.2 | 0.75 | 1.00 | 0.75 | 42.7 |
| 2 T1 | 4 | 0.0 | 4 | 0.0 | 0.121 | 103.9 | LOS F | 0.3 | 2.2 | 0.97 | 1.00 | 0.97 | 13.9 |
| Approach | 35 | 3.0 | 35 | 3.0 | 0.121 | 28.3 | LOS B | 0.3 | 2.2 | 0.77 | 1.00 | 0.77 | 36.3 |
| East: Major Road East |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 7 | 12.5 | 7 | 12.5 | 0.455 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 57.6 |
| $5 \quad \mathrm{~T} 1$ | 1734 | 3.0 | 1734 | 3.0 | 0.455 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.8 |
| 6 R2 | 1 | 100.0 | 1 | $\begin{array}{r} 100 . \\ 0 \end{array}$ | 0.001 | 6.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 50.6 |
| Approach | 1742 | 3.1 | 1742 | 3.1 | 0.455 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.8 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 12 R 2 | 7 | 0.0 | 7 | 0.0 | 0.103 | 39.9 | LOS C | 0.3 | 1.8 | 0.94 | 0.97 | 0.94 | 31.2 |
| 12u U | 3 | 0.0 | 3 | 0.0 | 0.103 | 40.5 | LOS C | 0.3 | 1.8 | 0.94 | 0.97 | 0.94 | 24.8 |
| Approach | 11 | 0.0 | 11 | 0.0 | 0.103 | 40.1 | NA | 0.3 | 1.8 | 0.94 | 0.97 | 0.94 | 29.5 |
| All Vehicles | 1787 | 3.1 | 1787 | 3.1 | 0.455 | 0.9 | NA | 0.3 | 2.2 | 0.02 | 0.03 | 0.02 | 58.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: X:117395 Doyalson Wyee RSLl07 Modelling Files\190614\17395_190614_Sc5 2038 Base.sip8

## MOVEMENT SUMMARY

$\nabla$ Site: 105 [Pacific Hwy-Wentworth Ave-Stage 2 (Median) NSW- FRI AM 2038 Base]

审审 Network: N101 [Pacific Hwy-Wentworth Ave-FRI AM 2038 Base]
Staged crossing Stage 2 (Median) at three-way intersection with 5 -lane major road.
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand Flows Arrival Flows |  |  |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Queue Vehicles Distance veh |  | Prop. Queued | Effective Stop Rate | Aver. Averag No. e Cycles Speed km/h |  |
| South: Median Storage Area |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 R2 | 4 | 0.0 | 4 | 0.0 | 0.010 | 6.6 | LOS A | 0.0 | 0.2 | 0.71 | 0.64 | 0.71 | 44.6 |
| Approach | 4 | 0.0 | 4 | 0.0 | 0.010 | 6.6 | LOS A | 0.0 | 0.2 | 0.71 | 0.64 | 0.71 | 44.6 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 T1 | 1291 | 9.0 | 1291 | 9.0 | 0.350 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| Approach | 1291 | 9.0 | 1291 | 9.0 | 0.350 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| All Vehicles | 1295 | 9.0 | 1295 | 9.0 | 0.350 | 0.0 | NA | 0.0 | 0.2 | 0.00 | 0.00 | 0.00 | 59.9 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

STIOF Site: 106 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road)-FRI PM 2038 Base ]

审禹 Network: N101 [Pacific Hwy-Wentworth Ave-FRI PM 2038 Base]
Staged crossing Stage 1 (Minor Road) at three-way intersection with 5 -lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand <br> Total veh/h | Flows | Arrival <br> Total veh/h | $\begin{gathered} \text { Flows } \\ \mathrm{HV} \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service |  | Back of eue Distance | Prop. Queued | Effective Stop Rate |  | Averag e Speed km/h |
| South: Minor Road (Stage 1) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 28 | 1.0 | 28 | 1.0 | 0.059 | 13.6 | LOS A | 0.2 | 1.4 | 0.61 | 0.99 | 0.61 | 45.7 |
| $2 \quad \mathrm{~T} 1$ | 3 | 0.0 | 3 | 0.0 | 0.034 | 45.1 | LOS D | 0.1 | 0.7 | 0.92 | 1.00 | 0.92 | 25.0 |
| Approach | 32 | 0.9 | 32 | 0.9 | 0.059 | 16.8 | LOS B | 0.2 | 1.4 | 0.64 | 0.99 | 0.64 | 43.2 |
| East: Major Road East |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 8 | 0.0 | 8 | 0.0 | 0.360 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 58.2 |
| $5 \quad \mathrm{~T} 1$ | 1360 | 4.0 | 1360 | 4.0 | 0.360 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| 6 R2 | 1 | 0.0 | 1 | 0.0 | 0.001 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 50.6 |
| Approach | 1369 | 4.0 | 1369 | 4.0 | 0.360 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.8 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 12 R 2 | 28 | 1.0 | 28 | 1.0 | 0.150 | 21.6 | LOS B | 0.4 | 3.0 | 0.87 | 0.95 | 0.87 | 39.0 |
| 12u U | 3 | 33.3 | 3 | 33.3 | 0.150 | 33.1 | LOS C | 0.4 | 3.0 | 0.87 | 0.95 | 0.87 | 29.8 |
| Approach | 32 | 4.2 | 32 | 4.2 | 0.150 | 22.7 | NA | 0.4 | 3.0 | 0.87 | 0.95 | 0.87 | 38.2 |
| All Vehicles | 1433 | 3.9 | 1433 | 3.9 | 0.360 | 1.0 | NA | 0.4 | 3.0 | 0.03 | 0.05 | 0.03 | 58.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

$\nabla$ Site: 107 [Pacific Hwy-Wentworth Ave-Stage 2 (Median) NSW- FRI PM 2038 Base]

审禹 Network: N101 [Pacific Hwy-Wentworth Ave-FRI PM 2038 Base]
Staged crossing Stage 2 (Median) at three-way intersection with 5 -lane major road.
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand Flows Arrival Flows |  |  |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Queue Vehicles Distance veh |  | Prop. Queued | Effective Stop Rate | Aver. Averag No. e Cycles Speed km/h |  |
| South: Median Storage Area |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 R2 | 3 | 0.0 | 3 | 0.0 | 0.018 | 18.5 | LOS B | 0.1 | 0.3 | 0.88 | 0.88 | 0.88 | 34.7 |
| Approach | 3 | 0.0 | 3 | 0.0 | 0.018 | 18.5 | LOS B | 0.1 | 0.3 | 0.88 | 0.88 | 0.88 | 34.7 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 T1 | 1919 | 3.0 | 1919 | 3.0 | 0.502 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.8 |
| Approach | 1919 | 3.0 | 1919 | 3.0 | 0.502 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.8 |
| All Vehicles | 1922 | 3.0 | 1922 | 3.0 | 0.502 | 0.1 | NA | 0.1 | 0.3 | 0.00 | 0.00 | 0.00 | 59.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

siof Site: 108 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road)-SAT 2038 Base]

审审 Network: N101 [Pacific Hwy-Wentworth Ave-SAT 2038

Staged crossing Stage 1 (Minor Road) at three-way intersection with 5-lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand <br> Total veh/h | HV HV \% | Arrival Total veh/h | ows <br> HV $\%$ | Deg. Satn v/c | Average Delay sec | Level of Service |  | ack of ue Distance | Prop. Queued | Effective Stop Rate | Aver. No. Cycles $\qquad$ | Averag Speed km/h |
| South: Minor Road (Stage 1) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 37 | 1.0 | 37 | 1.0 | 0.081 | 14.2 | LOS A | 0.3 | 1.9 | 0.64 | 1.00 | 0.64 | 45.4 |
| 2 T 1 | 11 | 0.0 | 11 | 0.0 | 0.129 | 51.1 | LOS D | 0.4 | 2.6 | 0.93 | 1.00 | 0.93 | 23.1 |
| Approach | 47 | 0.8 | 47 | 0.8 | 0.129 | 22.4 | LOS B | 0.4 | 2.6 | 0.70 | 1.00 | 0.70 | 39.2 |
| East: Major Road East |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 11 | 0.0 | 11 | 0.0 | 0.375 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 58.2 |
| $5 \quad \mathrm{~T} 1$ | 1422 | 3.0 | 1422 | 3.0 | 0.375 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.8 |
| 6 R2 | 1 | 0.0 | 1 | 0.0 | 0.001 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 50.6 |
| Approach | 1434 | 3.0 | 1434 | 3.0 | 0.375 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.8 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 12 R 2 | 14 | 0.0 | 14 | 0.0 | 0.073 | 22.8 | LOS B | 0.2 | 1.4 | 0.87 | 0.95 | 0.87 | 38.9 |
| 12u U | 1 | 0.0 | 1 | 0.0 | 0.073 | 23.7 | LOS B | 0.2 | 1.4 | 0.87 | 0.95 | 0.87 | 32.5 |
| Approach | 15 | 0.0 | 15 | 0.0 | 0.073 | 22.9 | NA | 0.2 | 1.4 | 0.87 | 0.95 | 0.87 | 38.5 |
| All Vehicles | 1496 | 2.9 | 1496 | 2.9 | 0.375 | 1.0 | NA | 0.4 | 2.6 | 0.03 | 0.05 | 0.03 | 58.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

$\nabla$ Site: 109 [Pacific Hwy-Wentworth Ave-Stage 2 (Median)
NSW- SAT 2038 Base]
审审 Network: N101 [Pacific Hwy-Wentworth Ave-SAT 2038

Base]
Staged crossing Stage 2 (Median) at three-way intersection with 5 -lane major road.
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | Demand Flows Arrival Flows |  |  |  | Deg. Satn v/c | Average Delay | Level of Service | 95\% Back of Queue Vehicles Distance veh m |  | Prop. Queued | Effective Stop Rate | Aver. Averag No. <br> Cycles Speed km/h |  |
|  | Total | HV | Total | HV |  |  |  |  |  |  |  |  |  |
|  | veh/h |  | veh/h | \% |  |  |  |  |  |  |  |  |  |
| South: Median Storage Area |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 R2 | 11 | 0.0 | 11 | 0.0 | 0.029 | 8.2 | LOS A | 0.1 | 0.5 | 0.75 | 0.75 | 0.75 | 43.0 |
| Approach | 11 | 0.0 | 11 | 0.0 | 0.029 | 8.2 | LOS A | 0.1 | 0.5 | 0.75 | 0.75 | 0.75 | 43.0 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 T1 | 1448 | 2.0 | 1448 | 2.0 | 0.376 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| Approach | 1448 | 2.0 | 1448 | 2.0 | 0.376 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| All Vehicles | 1459 | 2.0 | 1459 | 2.0 | 0.376 | 0.1 | NA | 0.1 | 0.5 | 0.01 | 0.01 | 0.01 | 59.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

## Site: 110 [Pacific Hwy-Rutley Rd -FRI AM 2038 Base]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 102 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Deman Total veh/h | $\begin{gathered} \text { Flows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Pacific Hwy - E |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 1589 | 4.0 | 0.418 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.8 |
| 6 | R2 | 220 | 6.0 | 0.438 | 30.8 | LOS C | 8.9 | 65.4 | 0.87 | 0.85 | 0.92 | 45.1 |
| Appr |  | 1809 | 4.2 | 0.438 | 3.8 | LOS A | 8.9 | 65.4 | 0.11 | 0.10 | 0.11 | 72.9 |
| North: Rutley Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 249 | 7.0 | 0.141 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 64.0 |
| 9 | R2 | 279 | 7.0 | 0.894 | 64.6 | LOS E | 16.3 | 120.6 | 1.00 | 0.96 | 1.37 | 31.7 |
| Appr | ach | 528 | 7.0 | 0.894 | 37.7 | LOS C | 16.3 | 120.6 | 0.53 | 0.79 | 0.72 | 41.7 |
| West: Pacific Hwy - W |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 125 | 10.0 | 0.127 | 13.3 | LOS A | 2.1 | 15.6 | 0.42 | 0.69 | 0.42 | 56.9 |
| 11 | T1 | 1031 | 7.0 | 0.613 | 22.8 | LOS B | 19.4 | 143.8 | 0.81 | 0.72 | 0.81 | 53.4 |
| Approach |  | 1156 | 7.3 | 0.613 | 21.7 | LOS B | 19.4 | 143.8 | 0.77 | 0.72 | 0.77 | 53.7 |
| All Ve | icles | 3494 | 5.7 | 0.894 | 14.9 | LOS B | 19.4 | 143.8 | 0.39 | 0.41 | 0.42 | 59.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

## Site: 111 [Pacific Hwy-Rutley Rd -FRI PM 2038 Base]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 102 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Pacific Hwy - E |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 1148 | 2.3 | 0.299 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| 6 | R2 | 279 | 2.0 | 0.528 | 41.6 | LOS C | 11.0 | 78.2 | 0.89 | 0.92 | 1.30 | 40.3 |
| Appr |  | 1427 | 2.2 | 0.528 | 8.2 | LOS A | 11.0 | 78.2 | 0.17 | 0.18 | 0.25 | 67.0 |
| North: Rutley Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 399 | 0.0 | 0.215 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 66.2 |
| 9 | R2 | 262 | 2.0 | 1.043 | 122.4 | LOS F | 22.1 | 157.4 | 1.00 | 1.17 | 2.02 | 21.0 |
| Approach |  | 661 | 0.8 | 1.043 | 53.1 | LOS D | 22.1 | 157.4 | 0.40 | 0.83 | 0.80 | 35.9 |
| West: Pacific Hwy - W |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 309 | 3.0 | 0.244 | 9.4 | LOS A | 2.7 | 19.6 | 0.32 | 0.69 | 0.32 | 62.4 |
| 11 | T1 | 1599 | 2.0 | 0.947 | 54.4 | LOS D | 51.9 | 369.7 | 1.00 | 1.14 | 1.33 | 36.5 |
| Appr |  | 1908 | 2.2 | 0.947 | 47.1 | LOS D | 51.9 | 369.7 | 0.89 | 1.06 | 1.16 | 39.2 |
| All V | icles | 3997 | 2.0 | 1.043 | 34.2 | LOS C | 51.9 | 369.7 | 0.55 | 0.71 | 0.78 | 45.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

## Site: 112 [Pacific Hwy-Rutley Rd -SAT 2038 Base]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 102 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Pacific Hwy - E |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 1176 | 1.6 | 0.305 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| 6 | R2 | 223 | 3.0 | 0.398 | 33.0 | LOS C | 8.6 | 61.8 | 0.84 | 0.87 | 1.02 | 44.3 |
| Appr |  | 1399 | 1.8 | 0.398 | 5.3 | LOS A | 8.6 | 61.8 | 0.13 | 0.14 | 0.16 | 70.8 |
| North: Rutley Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 229 | 1.7 | 0.125 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 65.7 |
| 9 | R2 | 200 | 1.9 | 0.795 | 58.4 | LOS E | 10.6 | 75.3 | 1.00 | 0.89 | 1.20 | 33.9 |
| Appr |  | 429 | 1.8 | 0.795 | 31.3 | LOS C | 10.6 | 75.3 | 0.47 | 0.73 | 0.56 | 45.9 |
| West: Pacific Hwy - W |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 236 | 1.2 | 0.210 | 11.3 | LOS A | 3.2 | 22.3 | 0.40 | 0.70 | 0.40 | 61.0 |
| 11 | T1 | 1248 | 2.3 | 0.736 | 25.3 | LOS B | 25.6 | 183.0 | 0.89 | 0.80 | 0.89 | 51.4 |
| Approach |  | 1484 | 2.1 | 0.736 | 23.1 | LOS B | 25.6 | 183.0 | 0.81 | 0.78 | 0.81 | 52.8 |
| All Ve | icles | 3313 | 2.0 | 0.795 | 16.6 | LOS B | 25.6 | 183.0 | 0.48 | 0.51 | 0.50 | 57.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

## Site: 101 [Pacific HIghway-Wyee Road-Scenic Drive -FRI AM 2038 +Stage 2]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 104 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { =lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 519 | 1.6 | 0.283 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.8 |
| 22 | T1 | 174 | 8.2 | 1.045 | 121.6 | LOS F | 15.1 | 113.3 | 1.00 | 1.32 | 2.13 | 19.9 |
| 23 | R2 | 277 | 3.0 | 1.283 | 314.3 | LOS F | 41.4 | 297.3 | 1.00 | 1.82 | 3.24 | 7.4 |
| Appro | ch | 969 | 3.2 | 1.283 | 114.6 | LOS F | 41.4 | 297.3 | 0.46 | 1.04 | 1.31 | 19.6 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 146 | 4.6 | 0.154 | 13.9 | LOS A | 2.1 | 15.3 | 0.54 | 0.73 | 0.54 | 47.5 |
| 25 | T1 | 2003 | 2.6 | 1.376 | 386.6 | LOS F | 177.1 | 1267.0 | 1.00 | 2.61 | 3.55 | 6.3 |
| 26 | R2 | 54 | 0.0 | 0.601 | 64.3 | LOS E | 2.9 | 20.5 | 1.00 | 0.77 | 1.10 | 25.3 |
| Appr |  | 2203 | 2.6 | 1.376 | 354.0 | LOS F | 177.1 | 1267.0 | 0.97 | 2.44 | 3.29 | 6.9 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 68 | 3.8 | 0.826 | 65.3 | LOS E | 11.2 | 83.0 | 1.00 | 1.04 | 1.79 | 25.8 |
| 28 | T1 | 149 | 7.8 | 0.826 | 59.6 | LOS E | 11.2 | 83.0 | 1.00 | 1.04 | 1.79 | 30.0 |
| 29 | R2 | 218 | 4.1 | 1.061 | 135.8 | LOS F | 19.8 | 143.3 | 1.00 | 1.34 | 2.15 | 18.7 |
| Appr |  | 436 | 5.3 | 1.061 | 98.6 | LOS F | 19.8 | 143.3 | 1.00 | 1.19 | 1.97 | 22.5 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 158 | 14.9 | 0.094 | 7.8 | LOSA | 0.0 | 0.0 | 0.00 | 0.59 | 0.00 | 61.8 |
| 31 | T1 | 1497 | 7.5 | 0.928 | 47.4 | LOS D | 50.0 | 372.9 | 0.96 | 1.06 | 1.23 | 33.5 |
| 32 | R2 | 272 | 8.7 | 1.077 | 147.7 | LOS F | 25.9 | 194.5 | 1.00 | 1.26 | 2.18 | 17.8 |
| Approach |  | 1926 | 8.3 | 1.077 | 58.3 | LOS E | 50.0 | 372.9 | 0.88 | 1.05 | 1.26 | 30.2 |
| All Vehicles |  | 5535 | 4.9 | 1.376 | 189.0 | LOS F | 177.1 | 1267.0 | 0.85 | 1.61 | 2.13 | 12.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^18]
## MOVEMENT SUMMARY

## Site: 102 [Pacific HIghway-Wyee Road-Scenic Drive -FRI PM 2038 +Stage 2]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 184 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Deman <br> Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 297 | 3.2 | 0.163 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.8 |
| 22 | T1 | 198 | 2.1 | 1.209 | 285.5 | LOS F | 34.5 | 245.8 | 1.00 | 1.48 | 2.16 | 10.2 |
| 23 | R2 | 328 | 1.6 | 1.545 | 571.4 | LOS F | 79.8 | 566.3 | 1.00 | 1.68 | 2.89 | 4.2 |
| Appro |  | 823 | 2.3 | 1.545 | 298.6 | LOS F | 79.8 | 566.3 | 0.64 | 1.22 | 1.67 | 9.0 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 287 | 2.0 | 0.370 | 27.6 | LOS B | 10.9 | 77.8 | 0.73 | 0.79 | 0.73 | 38.4 |
| 25 | T1 | 1440 | 3.5 | 1.549 | 567.1 | LOS F | 186.9 | 1347.0 | 1.00 | 2.20 | 2.86 | 4.3 |
| 26 | R2 | 82 | 0.0 | 0.542 | 96.7 | LOS F | 7.3 | 50.9 | 1.00 | 0.78 | 1.00 | 19.4 |
| Appro | ch | 1809 | 3.1 | 1.549 | 460.1 | LOS F | 186.9 | 1347.0 | 0.96 | 1.91 | 2.44 | 5.3 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 61 | 2.0 | 1.096 | 136.4 | LOS F | 39.3 | 277.8 | 1.00 | 1.06 | 1.76 | 10.2 |
| 28 | T1 | 229 | 1.1 | 1.096 | 130.7 | LOS F | 39.3 | 277.8 | 1.00 | 1.06 | 1.76 | 12.9 |
| 29 | R2 | 196 | 0.6 | 1.220 | 298.7 | LOS F | 34.5 | 242.8 | 1.00 | 1.35 | 2.19 | 9.9 |
| Appr |  | 486 | 1.0 | 1.220 | 199.1 | LOS F | 39.3 | 277.8 | 1.00 | 1.18 | 1.93 | 11.2 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 164 | 2.6 | 0.090 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 65.4 |
| 31 | T1 | 2137 | 1.7 | 1.369 | 403.0 | LOS F | 267.3 | 1898.1 | 1.00 | 2.08 | 2.44 | 6.1 |
| 32 | R2 | 636 | 1.1 | 1.314 | 344.5 | LOS F | 102.3 | 723.3 | 1.00 | 1.36 | 2.33 | 8.3 |
| Approach |  | 2937 | 1.6 | 1.369 | 368.3 | LOS F | 267.3 | 1898.1 | 0.94 | 1.85 | 2.28 | 7.0 |
| All Ve | icles | 6056 | 2.1 | 1.549 | 372.6 | LOS F | 267.3 | 1898.1 | 0.91 | 1.73 | 2.22 | 6.9 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^19]
## MOVEMENT SUMMARY

## Site: 103 [Pacific HIghway-Wyee Road-Scenic Drive -SAT 2038 +Stage 2]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 157 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Scenic Drive |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 396 | 1.0 | 0.215 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.9 |
| 22 | T1 | 175 | 3.1 | 1.081 | 176.1 | LOS F | 22.2 | 159.8 | 1.00 | 1.31 | 1.93 | 15.2 |
| 23 | R2 | 343 | 1.6 | 1.422 | 455.1 | LOS F | 71.2 | 504.9 | 1.00 | 1.69 | 2.90 | 5.2 |
| Appro |  | 914 | 1.6 | 1.422 | 207.0 | LOS F | 71.2 | 504.9 | 0.57 | 1.11 | 1.46 | 12.3 |
| NorthEast: Pacific Highway (north east) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 299 | 1.0 | 0.335 | 21.1 | LOS B | 8.6 | 61.1 | 0.66 | 0.78 | 0.66 | 42.3 |
| 25 | T1 | 1653 | 2.1 | 1.490 | 505.2 | LOS F | 193.5 | 1378.4 | 1.00 | 2.33 | 3.02 | 4.9 |
| 26 | R2 | 80 | 3.7 | 0.992 | 124.8 | LOS F | 7.8 | 56.6 | 1.00 | 0.99 | 1.71 | 16.2 |
| Appr |  | 2032 | 2.0 | 1.490 | 418.9 | LOS F | 193.5 | 1378.4 | 0.95 | 2.05 | 2.62 | 5.8 |
| NorthWest: Wyee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 96 | 2.6 | 1.083 | 128.1 | LOS F | 26.8 | 192.2 | 1.00 | 1.10 | 1.85 | 11.7 |
| 28 | T1 | 153 | 3.3 | 1.083 | 122.4 | LOS F | 26.8 | 192.2 | 1.00 | 1.10 | 1.85 | 14.7 |
| 29 | R2 | 164 | 2.3 | 1.083 | 179.8 | LOS F | 20.5 | 146.2 | 1.00 | 1.23 | 1.93 | 15.1 |
| Appr |  | 413 | 2.7 | 1.083 | 146.5 | LOS F | 26.8 | 192.2 | 1.00 | 1.15 | 1.88 | 14.2 |
| SouthWest: Pacific Highway (south west) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 156 | 5.5 | 0.087 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 64.5 |
| 31 | T1 | 1657 | 1.4 | 0.988 | 80.1 | LOS F | 88.8 | 629.3 | 0.93 | 1.10 | 1.24 | 23.9 |
| 32 | R2 | 414 | 1.4 | 1.070 | 135.4 | LOS F | 35.9 | 254.0 | 1.00 | 1.12 | 1.74 | 16.2 |
| Approach |  | 2226 | 1.7 | 1.070 | 85.3 | LOS F | 88.8 | 629.3 | 0.87 | 1.07 | 1.24 | 22.6 |
| All Ve | icles | 5584 | 1.8 | 1.490 | 231.1 | LOS F | 193.5 | 1378.4 | 0.86 | 1.44 | 1.83 | 10.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^20]
## MOVEMENT SUMMARY

siof Site: 104 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road)-FRI AM 2038 +Stage 2]

审审 Network: N101 [Pacific Hwy-Wentworth Ave-FRI AM 2038+Stage 2]
Staged crossing Stage 1 (Minor Road) at three-way intersection with 5-lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand <br> Total veh/h | Flows <br> HV <br> \% | Arriva <br> Total veh/h | $\begin{gathered} \text { Flows } \\ \mathrm{HV} \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Qu <br> Vehicles veh | of <br> tance <br> m | Prop. Queued | Effective Stop Rate |  | Averag <br> Speed <br> km/h |
| South: Minor Road (Stage 1) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 57 | 1.9 | 57 | 1.9 | 0.228 | 23.5 | LOS B | 0.8 | 5.7 | 0.83 | 1.02 | 0.90 | 39.4 |
| 2 T1 | 4 | 0.0 | 4 | 0.0 | 0.199 | 180.1 | LOS F | 0.5 | 3.6 | 0.98 | 1.01 | 1.01 | 8.8 |
| Approach | 61 | 1.7 | 61 | 1.7 | 0.228 | 34.3 | LOS C | 0.8 | 5.7 | 0.84 | 1.02 | 0.91 | 33.8 |
| East: Major Road East |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 9 | 11.1 | 9 | 11.1 | 0.560 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 57.3 |
| $5 \quad \mathrm{~T} 1$ | 2146 | 2.1 | 2146 | 2.1 | 0.560 | 0.1 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.8 |
| 6 R2 | 1 | 100.0 | 1 | $\begin{array}{r} 100 . \\ 0 \end{array}$ | 0.001 | 6.4 | LOSA | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 48.2 |
| Approach | 2157 |  | 2157 | 2.1 | 0.560 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.7 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 12 R 2 | 13 | 0.0 | 13 | 0.0 | 0.266 | 72.8 | LOS F | 0.7 | 4.7 | 0.97 | 1.00 | 1.03 | 22.7 |
| 12u U | 3 | 0.0 | 3 | 0.0 | 0.266 | 73.0 | LOS F | 0.7 | 4.7 | 0.97 | 1.00 | 1.03 | 17.0 |
| Approach | 16 | 0.0 | 16 | 0.0 | 0.266 | 72.9 | NA | 0.7 | 4.7 | 0.97 | 1.00 | 1.03 | 21.7 |
| All Vehicles | 2234 | 2.1 | 2234 | 2.1 | 0.560 | 1.6 | NA | 0.8 | 5.7 | 0.03 | 0.04 | 0.03 | 57.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: TTPP - THE TRANSPORT PLANNING PARTNERSHIP | Processed: Monday, 17 June 2019 1:19:57 PM
Project: X:\17395 Doyalson Wyee RSLl07 Modelling Files\190614|17395_190614_Sc6 2038 +Stage 2.sip8

## MOVEMENT SUMMARY

$\nabla$ Site: 105 [Pacific Hwy-Wentworth Ave-Stage 2 (Median)
NSW- FRI AM 2038 +Stage 2]
审审 Network: N101 [Pacific Hwy-Wentworth Ave-FRI AM 2038+Stage 2]
Staged crossing Stage 2 (Median) at three-way intersection with 5 -lane major road
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand Flows Arrival Flows |  |  |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Queue Vehicles Distance veh |  | Prop. Queued | Effective Stop Rate | Aver. Averag No. e Cycles Speed km/h |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | veh/h |  | Total | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ |  |  |  |  |  |  |  |  |  |
| South: Median Storage Area |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 R2 | 4 | 0.0 | 4 | 0.0 | 0.017 | 12.7 | LOS A | 0.0 | 0.3 | 0.83 | 0.83 | 0.83 | 35.2 |
| Approach | 4 | 0.0 | 4 | 0.0 | 0.017 | 12.7 | LOS A | 0.0 | 0.3 | 0.83 | 0.83 | 0.83 | 35.2 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 T1 | 1681 | 6.8 | 1681 | 6.8 | 0.450 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| Approach | 1681 | 6.8 | 1681 | 6.8 | 0.450 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| All Vehicles | 1685 | 6.8 | 1685 | 6.8 | 0.450 | 0.1 | NA | 0.0 | 0.3 | 0.00 | 0.00 | 0.00 | 59.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

shivi Site: 106 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road)-FRI PM 2038 +Stage 2]

审禹 Network: N101 [Pacific Hwy-Wentworth Ave-FRI PM 2038+Stage 2]
Staged crossing Stage 1 (Minor Road) at three-way intersection with 5-lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand <br> Total veh/h | Flows HV \% | Arrival Total veh/h | $\begin{gathered} =l o w s \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service |  | ack of ue Distance | Prop. Queued | Effective Stop Rate | Aver. No. Cycles $\qquad$ | Averag Speed km/h |
| South: Minor Road (Stage 1) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 35 | 3.0 | 35 | 3.0 | 0.093 | 16.7 | LOS B | 0.3 | 2.3 | 0.70 | 1.00 | 0.70 | 43.5 |
| 2 T 1 | 3 | 0.0 | 3 | 0.0 | 0.058 | 70.8 | LOS F | 0.2 | 1.1 | 0.95 | 1.00 | 0.95 | 18.5 |
| Approach | 38 | 2.8 | 38 | 2.8 | 0.093 | 21.2 | LOS B | 0.3 | 2.3 | 0.73 | 1.00 | 0.73 | 40.3 |
| East: Major Road East |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 18 | 0.0 | 18 | 0.0 | 0.461 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 57.9 |
| $5 \quad \mathrm{~T} 1$ | 1753 | 2.2 | 1753 | 2.2 | 0.461 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 59.8 |
| 6 R2 | 1 | 0.0 | 1 | 0.0 | 0.001 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 48.2 |
| Approach | 1772 | 2.2 | 1772 | 2.2 | 0.461 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 59.7 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 12 R 2 | 57 | 3.7 | 57 | 3.7 | 0.461 | 42.5 | LOS C | 1.4 | 10.4 | 0.95 | 1.03 | 1.16 | 30.0 |
| 12u U | 3 | 33.3 | 3 | 33.3 | 0.461 | 61.0 | LOS E | 1.4 | 10.4 | 0.95 | 1.03 | 1.16 | 22.2 |
| Approach | 60 | 5.3 | 60 | 5.3 | 0.461 | 43.4 | NA | 1.4 | 10.4 | 0.95 | 1.03 | 1.16 | 29.7 |
| All Vehicles | 1869 | 2.3 | 1869 | 2.3 | 0.461 | 1.9 | NA | 1.4 | 10.4 | 0.05 | 0.06 | 0.05 | 57.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

$\nabla$ Site: 107 [Pacific Hwy-Wentworth Ave-Stage 2 (Median) NSW- FRI PM 2038 +Stage 2]

㿾审 Network: N101 [Pacific Hwy-Wentworth Ave-FRI PM 2038+Stage 2]
Staged crossing Stage 2 (Median) at three-way intersection with 5 -lane major road
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)


Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

siof Site: 108 [Pacific Hwy-Wentworth Ave-Stage 1 (Minor Road)-SAT 2038 +Stage 2]

审审 Network: N101 [Pacific Hwy-Wentworth Ave-SAT 2038 +Stage 2]
Staged crossing Stage 1 (Minor Road) at three-way intersection with 5-lane major road. Major road turn lane is treated as a full-length lane.
Site Category: (None)
Stop (Two-Way)


Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

$\nabla$ Site: 109 [Pacific Hwy-Wentworth Ave-Stage 2 (Median) NSW- SAT 2038 +Stage 2]

审审 Network: N101 [Pacific Hwy-Wentworth Ave-SAT 2038 +Stage 2]
Staged crossing Stage 2 (Median) at three-way intersection with 5-lane major road.
Give-way behaviour assumed at Stage 2.
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand <br> Total veh/h |  | Arriva <br> Total veh/h | ows <br> HV <br> \% | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Qu Vehicles veh | of <br> ance m | Prop. Queued | Effective <br> Stop <br> Rate | Aver. No. Cycles | Averag <br> Speed km/h |
| South: Median Storage Area |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 R2 | 11 | 0.0 | 11 | 0.0 | 0.072 | 22.1 | LOS B | 0.2 | 1.1 | 0.90 | 0.90 | 0.90 | 28.5 |
| Approach | 11 | 0.0 |  | 0.0 | 0.072 | 22.1 | LOS B | 0.2 | 1.1 | 0.90 | 0.90 | 0.90 | 28.5 |
| West: Major Road West |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 T1 | 2017 |  | 2017 | 1.1 | 0.521 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.8 |
| Approach | 2017 |  | 2017 | 1.1 | 0.521 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.8 |
| All Vehicles | 2027 |  | 2027 | 1.1 | 0.521 | 0.2 | NA | 0.2 | 1.1 | 0.00 | 0.00 | 0.00 | 59.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

## Site: 110 [Pacific Hwy-Rutley Rd -FRI AM 2038 +Stage 2]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 102 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{gathered} \text { Flows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Pacific Hwy - E |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 1722 | 2.9 | 0.450 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.8 |
| 6 | R2 | 220 | 5.7 | 0.462 | 35.5 | LOS C | 9.0 | 66.3 | 0.89 | 0.89 | 1.06 | 42.7 |
| Appro |  | 1942 | 3.3 | 0.462 | 4.1 | LOS A | 9.0 | 66.3 | 0.10 | 0.10 | 0.12 | 72.6 |
| North: Rutley Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 249 | 5.5 | 0.140 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 64.5 |
| 9 | R2 | 279 | 6.0 | 0.888 | 63.6 | LOS E | 16.1 | 118.5 | 1.00 | 0.96 | 1.35 | 32.0 |
| Appro |  | 528 | 5.8 | 0.888 | 37.2 | LOS C | 16.1 | 118.5 | 0.53 | 0.79 | 0.71 | 42.1 |
| West: Pacific Hwy - W |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 125 | 10.1 | 0.121 | 12.1 | LOS A | 1.8 | 13.5 | 0.40 | 0.69 | 0.40 | 58.0 |
| 11 | T1 | 1177 | 4.7 | 0.690 | 23.9 | LOS B | 23.3 | 169.4 | 0.86 | 0.77 | 0.86 | 52.5 |
| Approach |  | 1302 | 5.3 | 0.690 | 22.8 | LOS B | 23.3 | 169.4 | 0.81 | 0.76 | 0.81 | 53.0 |
| All Vehicles |  | 3773 | 4.3 | 0.888 | 15.2 | LOS B | 23.3 | 169.4 | 0.41 | 0.42 | 0.44 | 59.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

## Site: 111 [Pacific Hwy-Rutley Rd -FRI PM 2038 +Stage 2]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 102 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Pacific Hwy - E |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 1327 | 2.0 | 0.345 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.8 |
| 6 | R2 | 279 | 1.5 | 0.720 | 51.2 | LOS D | 12.7 | 89.8 | 0.99 | 0.98 | 1.53 | 36.5 |
| Appro | ch | 1606 | 1.9 | 0.720 | 8.9 | LOS A | 12.7 | 89.8 | 0.17 | 0.17 | 0.27 | 66.1 |
| North: Rutley Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 399 | 0.0 | 0.215 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 66.2 |
| 9 | R2 | 262 | 2.0 | 0.913 | 68.3 | LOS E | 15.7 | 111.8 | 1.00 | 0.98 | 1.44 | 31.1 |
| Appro |  | 661 | 0.8 | 0.913 | 31.6 | LOS C | 15.7 | 111.8 | 0.40 | 0.75 | 0.57 | 45.8 |
| West: Pacific Hwy - W |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 309 | 2.4 | 0.247 | 10.1 | LOS A | 2.9 | 20.7 | 0.37 | 0.70 | 0.37 | 61.9 |
| 11 | T1 | 1763 | 1.1 | 0.926 | 43.4 | LOS D | 53.1 | 375.0 | 0.99 | 1.08 | 1.22 | 41.0 |
| Approach |  | 2073 | 1.3 | 0.926 | 38.4 | LOS C | 53.1 | 375.0 | 0.90 | 1.02 | 1.09 | 43.2 |
| All Vehicles |  | 4340 | 1.5 | 0.926 | 26.5 | LOS B | 53.1 | 375.0 | 0.55 | 0.67 | 0.71 | 50.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

## Site: 112 [Pacific Hwy-Rutley Rd -SAT 2038 +Stage 2]

New Site
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 102 seconds (Site User-Given Phase Times)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand <br> Total veh/h |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Pacific Hwy - E |  |  |  |  |  |  |  |  |  |  |  |
| $5 \quad$ T1 | 1381 | 1.1 | 0.357 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.8 |
| 6 R2 | 223 | 2.4 | 0.415 | 37.6 | LOS C | 8.8 | 62.6 | 0.85 | 0.89 | 1.15 | 42.0 |
| Approach | 1604 | 1.2 | 0.415 | 5.3 | LOS A | 8.8 | 62.6 | 0.12 | 0.12 | 0.16 | 70.9 |
| North: Rutley Rd |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 229 | 1.4 | 0.125 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 65.8 |
| 9 R2 | 200 | 1.6 | 0.793 | 58.3 | LOS E | 10.6 | 75.0 | 1.00 | 0.88 | 1.20 | 34.0 |
| Approach | 429 | 1.5 | 0.793 | 31.2 | LOS C | 10.6 | 75.0 | 0.47 | 0.73 | 0.56 | 45.9 |
| West: Pacific Hwy - W |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 236 | 0.9 | 0.192 | 9.8 | LOS A | 2.2 | 15.7 | 0.34 | 0.69 | 0.34 | 62.6 |
| 11 T1 | 1466 | 1.4 | 0.860 | 34.3 | LOS C | 36.9 | 261.4 | 0.97 | 0.96 | 1.08 | 45.6 |
| Approach | 1702 | 1.4 | 0.860 | 30.9 | LOS C | 36.9 | 261.4 | 0.88 | 0.92 | 0.98 | 47.4 |
| All Vehicles | 3736 | 1.3 | 0.860 | 20.0 | LOS B | 36.9 | 261.4 | 0.51 | 0.56 | 0.58 | 55.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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transport planning

## Appendix D

## Concept Signalised Intersection




[^0]:    Source: Roads and Maritime Services Cycleways Finder (last updated 27/02/18)

[^1]:    Source: Urbis

[^2]:    ${ }^{1}$ An average passing by rate of $50 \%$ has been assumed, however, the draft guide references various rates for each surveyed food outlet including $51 \%$ for McDonalds, $43 \%$ for KFC, $54 \%$ for Hungry Jacks.

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